

**महात्मा गांधी राष्ट्रीय ग्रामीण
रोजगार हमी योजनेतून गावातील
अंतर्गत रस्ते घेण्याबाबत.**

**महाराष्ट्र शासन
नियोजन विभाग
शासन परिपत्रक क्र. मग्रारो - २०११/प्र.क्र.११७/रोहयो- १० अ
मंत्रालय, मुंबई- ४०० ०३२
दिनांक :- १७ फेब्रुवारी, २०१२**

परिपत्रक :-

महात्मा गांधी राष्ट्रीय ग्रामीण रोजगार हमी अधिनियमाच्या परिशिष्ट-१ नुसार गावातील अंतर्गत रस्ते या योजनेतून घेता येतात. केंद्र शासनाने अधिनियमाच्या अंमलबजावणीबाबत मार्गदर्शक सुचनांमध्ये सुधारणा करून सुधारित आदेश पारित केलेले आहेत. दिनांक १८ ऑक्टोबर, २०११ व ६ जानेवारी, २०१२ च्या परिपत्रकांद्वारे परिच्छेद ६.१.१ (आठ) मध्ये सुधारणा करण्यात आल्या आहेत. केंद्र शासनाने या दोन परिपत्रकांद्वारे केलेल्या सुधारणांप्रमाणे परिच्छेद ६.१.१ (आठ) ची इंग्रजी प्रत सोबत जोडली आहे. ह्या दोन्ही परिपत्रकांमधील सुचना एकत्रित करून अंमलबजावणी करण्यासाठी सदर शासन परिपत्रक निर्गमित करण्यात येत आहे. तरी, दिनांक १८.१०.२०११ व दिनांक ६.१.२०१२ मधील सूचना एकत्रित करून खालीलप्रमाणे कार्यवाही करण्यात यावी.

६.१.१ (आठ) :- सर्व हंगामात वापरता येण्याजोगे गावातील अंतर्गत रस्त्यांची १) एका गावाला दुसऱ्या गावाशी जोडणारे किंवा मोठ्या रस्त्याशी जोडणारे रस्ते व २) त्या गावातील गावठाणांना जोडणारे अंतर्गत रस्ते या प्रमाणे वर्गवारी करावी.

वरील १) प्रमाणे रस्ता मग्रारोहयोंतर्गत घेताना पंतप्रधान ग्राम सडक योजनेंतर्गत नियोजित रस्ता त्यामध्ये घेऊ नये. तसेच या रस्त्याचे बांधकाम करताना रस्त्याखालील नाले (drainage) व त्यावरील कठडे याचे आवश्यकतेप्रमाणे बांधकाम करावे.

गावातील अंतर्गत रस्ते बांधताना खालील बाबी विचारात घ्याव्यात.

अ) गावातील अंतर्गत रस्त्यांचे बांधकाम शक्यतो स्थानिक उपलब्ध साहित्य आणि त्या प्रदेशातील रस्त्यांसाठी आवश्यकतेप्रमाणे दगडाचे तुकडे किंवा विटांचे तुकडे किंवा सिमेंट काँक्रीट किंवा सिमेंट काँक्रीट ठोकळे (Interlock block) वापरता येतील.

ब) रस्त्यांच्या बांधकामासाठी वापरावयाच्या साहित्याचा दर्जा व सिमेंट काँक्रीटच्या तयार ठोकळ्यांचे रस्ते बनवताना सिमेंट काँक्रीटचे तयार ठोकळे बसवण्यासाठी वापरावयाची कार्यपद्धती व त्यांचा दर्जा याबाबत राज्य शासनाने कार्यपद्धती निश्चित करावी.

क) या योजनेतून घ्यावयाच्या रस्त्यांची रुंदी २.५ मिटर पेक्षा जास्त असू नये.

ड) अंतर्गत रस्ते घेताना त्यासोबत नाले (Drainage) व्यवस्थाही आवश्यकतेप्रमाणे करावी.

इ) अनुसूचित जाती / जमातीच्या नागरिकांच्या वस्तीसाठी प्राधान्याने रास्त्यांची सोय उपलब्ध करून द्यावी.

फ) अकुशल व कुशल (Material cost) प्रमाण ६०:४० हे ग्रामपंचायत पातळीवर ठेवावे.

केंद्र शासनाच्या वरील प्रमाणे सुधारीत आदेशाप्रमाणे गावातील अंतर्गत रस्ते घेण्याची कार्यवाही करावी. वरील 'ब' मध्ये केंद्र शासनाने दिलेल्या सूचनेनुसार रस्त्यांच्या बांधकामासाठी वापरावयाच्या साहित्याचा दर्जा तसेच सिमेंट काँक्रीटचे ठोकळे वापरावयाचे झाल्यास त्यांचा दर्जा व ते बसविण्यासाठी कार्यपद्धती याबाबत जिल्हा स्तरावर समिती जिल्हाधिकारी यांनी नियुक्त करावी. सदर समितीने स्थानीक परिस्थिती लक्षात घेवून तसेच भारतीय रोड काँग्रेसने (Indian Road Congress) सिमेंट काँक्रीट ठोकळ्यांच्या वापराबाबत संदर्भ क्र.आयआरसी / एसपी/ ६३-२००४ अन्वये दिलेल्या मार्गदर्शक सुचनांचे पालन रस्त्यांच्या बांधकामाबाबत करण्याच्या अनुषंगाने मार्गदर्शक सुचना द्याव्यात. (आयआरसी /एसपी/६३-२००४ अन्वये दिलेल्या मार्गदर्शक सुचनांची प्रत महाराष्ट्र शासनाच्या वेबसाईटवरून या शासन परिपत्रकासोबत उपलब्ध करून देण्यात आली आहे.)

हे परिपत्रक महाराष्ट्र शासनाच्या www.maharashtra.gov.in वेबसाईटवर उपलब्ध करण्यात आला आहे. त्याचा संगणक क्र.२०१२०२१७११०८२६२२६००१ असा आहे.


(आर. विमला)

उप सचिव , नियोजन विभाग
महाराष्ट्र शासन

17/2/2012

प्रति,

सर्व जिल्हाधिकारी,
सर्व मुख्य कार्यकारी अधिकारी, जिल्हा परिषद,

**Amendment in para 6.1.1 (viii) of MGNREGA Operational Guidelines
related to rural connectivity.**

(Vide Govt. Of. India, Ministry of Rural Development Circular dated 18 Oct, 2011 and amendment dated 6 January, 2012)

(viii) Rural connectivity to provide all-weather access comprises of two categories of roads (1) Roads that connect a village to another village or to a main road and (2) Roads that are internal (within the residential areas of villages).

As regards category (1) above, care should be taken not to take up roads included in the PMGSY network under Mahatma Gandhi NREGA and construction of such roads should include culverts / cross drainage where necessary,

As regards internal roads, the following is to be observed:

(a) As per local availability of material and requirement of the area for the construction of village internal streets (not roads connecting habitations) stone kharanja or brick kharanja or cement concrete or cement concrete interlocking blocks may be used.

(b) The state Government will lay down procedure to ensure that the quality of material being procured & the process used for laying precast cement concrete blocks in constructing cement concrete interlocking block roads follow appropriate standards (IRC reference **IRC SP : 63-2004** " Guidelines for the use of Interlocking Concrete Block Pavement" may be referred to, and they are being uploaded on Government of Maharashtra website " www.maharashtra.gov.in ")

(c) The width of such internal roads taken up under this provision should not be more than 2.5 meter.

(d) Internal Roads within the village area should be taken up alongwith proper drainage arrangement.

(e) Priority should be given to roads that give access to SC/ST habitations.

(f) Labour material ratio 60:40 should be maintained at the Gram Panchayat level.

GUIDELINES FOR THE USE OF INTERLOCKING CONCRETE BLOCK PAVEMENT

Published by

THE INDIAN ROADS CONGRESS

Jamnagar House, Shahjahan Road,

New Delhi-110011

2004

Price Rs. 100/-
(Plus packing and postage)

IRC:SP:63-2004

First Published : October, 2004
Reprinted : July, 2006

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Printed at Gopsons Papers Ltd., Noida
(500 copies)

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PERSONNEL OF THE HIGHWAYS SPECIFICATIONS AND STANDARDS COMMITTEE
(As on 22.5.2004)

- | | | |
|----|---|---|
| 1. | Indu Prakash*
(Convenor) | Director General (Road Development) & Spl. Secretary to the Govt. of India, Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 2. | G. Sharan
(Co-Convenor) | Chief Engineer (R&B) S&R, Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 3. | The Chief Engineer
(R&B) S&R
(Member-Secretary) | (G. Sharan) Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |

Member

- | | | |
|-----|--------------------|---|
| 4. | A.P. Bahadur | Chief Engineer, Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 5. | P.K. Chakrabarty | Chief General Manager (NS), National Highways Authority of India, Plot No. G/5-6, Sector 10, Dwarka, New Delhi-110045 |
| 6. | P.K. Datta | Executive Director, Consulting Engg. Services (I) Pvt. Ltd., 57, Nehru Place, New Delhi-110019 |
| 7. | J.P. Desai | Sr. Vice-President (Tech. Ser.), Gujarat Ambuja Cements Ltd., Ambuja House, Ishwarbhuwan Road, Navrangpura, Ahmedabad-380009 |
| 8. | Dr. S.L. Dhingra | Professor, Transportation System, Civil Engg. Department, Indian Institute of Technology, Mumbai, Powai, Mumbai-400076 |
| 9. | D.P. Gupta | DG(RD) (Retd.), E-44, Greater Kailash (Part I) Enclave, New Delhi-110048 |
| 10. | S.K. Gupta | Chief Engineer, PWD, Almora |
| 11. | R.K. Jain | Chief Engineer (Retd.), House No. 452, Sector 14, Sonapat-131001 |
| 12. | Dr. S.S. Jain | Professor & Coordinator (COTE), Deptt. of Civil Engg., Indian Institute of Technology, Roorkee, Roorkee-247667 |
| 13. | Dr. L.R. Kadiyali | Chief Executive, L.R. Kadiyali & Associates, X-15 (First Floor), Hauz Khas, New Delhi-110016 |
| 14. | Prabha Kant Katare | Joint Director (PI), National Rural Road Dev. Agency (Min. of Rural Dev.) NBCC Tower, 5th Floor, Bhikaji Cama Place, New Delhi-110066 |
| 15. | J.B. Mathur | Chief Engineer (Retd.), H. No. 77, 1st Floor, Sector 15A, Distt. Gautam Budh Nagar, Noida-201301 |

* ADG(R) being not in position, the meeting was presided by Shri Indu Prakash, DG(RD) & Spl. Secretary to the Govt. of India, MORT&H

16. H.L. Mina Chief Engineer-cum-Addl. Secy. to the Govt. of Rajasthan, P.W.D., Jacob Road, Jaipur-302006
17. S.S. Momin Secretary (Works), Maharashtra P.W.D., Mantralaya, Mumbai-400032
18. A.B. Pawar Secretary (Works) (Retd.), C-58, Abhimanshree Housing Society, Off Pashan Road, Pune-411008
19. Dr. Gopal Ranjan Director, College of Engg., Roorkee, Post Box No. 27, K.M. Roorkee-Hardwar Road, Vardhman Puram, Roorkee-247667
20. S.S. Rathore Secretary to the Govt. of Gujarat, R&B Department, Block No. 14/1, Sardar Bhavan, Sachivalaya, Gandhinagar-382010
21. Arghya Pradip Saha Sr. Consultant, M-504, Habitat (Highway) CGHS, B-19, Vasundhara Enclave, Delhi
22. S.C. Sharma DG(RD) & AS, MORT&H (Retd.), 175, Vigyapan Lok, 15, Mayur Vihar Phase-I Extn. (Near Samachar Apartments), Delhi-110091
23. Prof. P.K. Sikdar Director, Central Road Research Institute, P.O. CRRI, Delhi-Mathura Road, New Delhi-110020
24. Dr. C.K. Singh Engineer-in-Chief-cum-Addl. Comm-cum.-Spl. Secy. (Retd.), House No. M-10 (D.S.) Hermu Housing Colony, Main Hermu Road, Ranchi (Jharkhand)
25. Nirmal Ji Singh Member (Tech.), National Highways Authority of India, Plot No. G/5-6, Sector 10, Dwarka, New Delhi-110045
26. A.V. Sinha Chief General Manager, National Highways Authority of India, Plot No. G/5-6, Sector 10 Dwarka, New Delhi-110045
27. N.K. Sinha DG(RD & SS, MORT&H (Retd.), G-1365, Ground Floor, Chitranjan Park, New Delhi-110019
28. V.K. Sinha Chief Engineer, Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001
29. K.K. Sarin DG(RD) & AS, MOST (Retd.), S-108, Panchshila Park, New Delhi-110017
30. T.P. Velayudhan Addl. D.G.B.R., Directorate General Border Roads, Seema Sadak Bhavan, Ring Road, Delhi Cantt., New Delhi-110010
31. Maj. V.C. Verma Executive Director-Marketing, Oriental Structural Engrs. Pvt. Ltd., 21, Commercial Complex, Malcha Marg, Diplomatic Encl., New Delhi-110021
32. The Chief Engineer (NH) (B. Prabhakar Rao), R&B Department, Errum Manzil, Hyderabad-500082

- | | |
|--------------------------------|---|
| 33. The Chief Engineer (Plg.) | (S.B. Basu), Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 34. The Chief Engineer (Mech.) | (V.K. Sachdev), Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 35. The Chief Engineer (Mech.) | PWD, G Block, 4th Floor, Writers' Building, Kolkata-700001 |
| 36. The Chief Engineer (NH) | (Ratnakar Dash), Sachivalaya Marg, Unit IV, Bhubaneswar-751001 Distt. Khurdha (Orissa) |
| 37. The Engineer-in-Chief | U.P. P.W.D., 96, M.G. Road, Lucknow-226001 |
| 38. The Chief Engineer | National Highways, PWD Annexe, K.R. Circle, Bangalore-560001 |

Ex-Officio Members

- | | |
|---|---|
| 39. President,
Indian Road Congress | (S.S. Momin), Secretary (Works), PWD Sachivalaya, Mumbai-400032 |
| 40. The Director General
(Road Development)
& Special Secretary | (Indu Prakash), Ministry of Road Transport & Highways, Transport Bhavan, New Delhi-110001 |
| 41. Secretary,
Indian Roads Congress | (R.S. Sharma), Indian Roads Congress, Jamnagar House, New Delhi-110011 |

Corresponding Members

- | | |
|----------------------|---|
| 1. M.K. Agarwal | Engineer-in-Chief, Haryana PWD (Retd.) House No. 40, Sector 16, Panchkula-134113 |
| 2. Dr. C.E.G. Justo | Emeritus Fellow, 334, 25th Cross, 14th Main, Banashankari, 2nd Stage, Bangalore-560070 |
| 3. M.D. Khattar | Executive Director, Hindustan Construction Co. Ltd., Hicon House, Lal Bahadur Shastri Marg, Vikhroli (W), Mumbai-400083 |
| 4. Sunny C. Madathil | Director (Project), Bhagheeratha Engg. Ltd., 132, Panampilly Avenue, Cochin-682036 |
| 5. N.V. Merani | Principal Secretary, Maharashtra PWD (Retd.), A-47/1344, Adarsh Nagar, Worli, Mumbai-400025 |

GUIDELINES FOR THE USE OF INTERLOCKING CONCRETE BLOCK PAVEMENTS

Background

The Rigid Pavement Committee (H-5) in its meeting held on the 28th November, 2003 discussed the draft Guidelines for the Use of Interlocking Concrete Block Pavements and desired to recirculate the modified document in light of the suggestions made by the members to improve the document. Accordingly, the modified document was circulated and discussed during the meeting of H-5 Committee (Personnel given below) held on the 8th March, 2004 and the document was approved with certain modifications :

Rigid Pavement Committee (H-5)

Dr. L.R. Kadiyali	Convenor
The CE (R&B) S&R, MORT&H (G. Sharan)	Co-Convenor
M.C. Venkatesha	Member-Secretary

Members

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Dr. B.B. Pandey	A Rep. of MSRDC (P.D. Kulkarni)
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Secretary, IRC (R.S. Sharma)	

Corresponding Members

K.B. Bhaumik	Prof. K.V. Krishna Rao
D.C. De	A.U. Ravi Shankar
Dr. (Mrs.) Vandana Tare	

The draft was discussed by the Highways Specifications and Standards Committee during its meeting held on the 22nd May, 2004 and the draft was approved subject to modifications in light of the comments made by its members. The modified document as received from the Convenor, H-5 Committee was placed before the Executive Committee in its meeting held on 25th May, 2004. The Executive Committee considered the Guidelines for being placed before the Council. The Council in its 172nd Meeting held at Nainital (Uttaranchal) on the 12th June, 2004 approved the document for publication subject to modification in light of the comments/suggestions given by the participants. The document has been modified suitably by Dr. L.R. Kadiyali, Convenor, H-5 Committee.

1. SCOPE

Interlocking Concrete Block Pavements have been extensively used in a number of countries for quite sometime. Considering their advantages and potential for use, the guidelines have been prepared for the design and construction of such pavements, giving the suggested applications, design catalogues, construction practices and specifications for their use.

2. APPLICATIONS

2.1. Interlocking Concrete Block Pavements have been found to have applications in several situations. Such as :

1. Footpaths and Side-walks
2. Cycle Tracks
3. Residential streets
4. Car Parks
5. Fuel Stations
6. Rural Roads through villages
7. Highway Rest Areas
8. Toll Plaza
9. Bus Depots
10. Approaches to Railway Level Crossings
11. Intersections
12. City Streets
13. Truck Parking Areas
14. Industrial floors
15. Urban Sections of Highways
16. Road Repairs during Monsoon
17. Container Depots
18. Port Wharf and Roads
19. Roads in high altitude areas

2.2. Advantages and Limitations of Interlocking Concrete Block Pavements.

2.2.1. Advantages

- (i) Since the blocks are prepared in the factory, they are of a very high quality, thus avoiding the difficulties encountered in quality control in the field.
- (ii) Concrete block pavements restrict the speed of vehicles to about 60 km per hour, which is an advantage in city streets and intersections.
- (iii) Because of the rough surface, these pavements are skid-resistant.
- (iv) The block pavements are ideal for intersections where speeds have to be restricted and cornering stresses are high.
- (v) The digging and reinstatement of trenches for repairs to utilities is easier in the case of block pavement.
- (vi) These pavements are unaffected by the spillage of oil from vehicles, and are ideal for bus stops, bus depots and parking areas.
- (vii) They are preferred in heavily loaded areas like container depots and ports as they can be very well designed to withstand the high stresses induced there.
- (viii) In India, the laying of concrete block pavements can be achieved at a low cost because of the availability of cheap labour.
- (ix) Since the concrete blocks are grey in colour, they reflect light better than the black bituminous pavements, thus bringing down the cost of street lighting.
- (x) The cost of maintenance is much lower than a bituminous surface.
- (xi) Block pavement does not need in-situ curing and so can be opened to traffic soon after completion of construction.
- (xii) Construction of block pavement is simple and labour-intensive, and can be done using simple compaction equipment.
- (xiii) Maintenance of block pavement is simple and easy. Also, the need for frequency of maintenance is low as compared to bituminous pavement.
- (xiv) Structurally round blocks can be recycled many times over.
- (xv) Unlike concrete pavements, block pavement does not exhibit very deterioratory effect due to thermal expansion and contraction, and are free from the cracking phenomenon.
- (xvi) Use of permeable block pavement in cities and towns can help replenish depleting underground sources of water, filter pollutants before they reach open water sources, help reduce storm water runoff and decrease the quantum of drainage structures.

2.2.2. Limitations

- (i) Concrete block pavements cannot be used for high speed facilities.
- (ii) The riding quality is reasonably good for low-speed traffic, but is inferior to that observed on a machine laid bituminous or concrete pavement.

- (iii) The noise generated is high, 5-8 dB (A) higher than bituminous surfaces.
- (iv) A very good attention to pavement drainage is needed because the water can seep through the joints.

3. TYPES AND SHAPES OF BLOCKS

The blocks can be interlocking horizontally and vertically, as shown in Fig 1.

Present day interlocking blocks have evolved in shape after observing their performance. The three phases in the evolution of the shape of the blocks are shown in Fig. 2.

The rectangular shape shown in Fig. 2(i) is the shape which was intended for imitating the stone set blocks. The shape shown in Fig. 2(ii) is an improved version with many dentated faces for better contact between adjoining blocks thus enhancing the interlocking effect and friction between them. This helps in increasing the shear strength of the block system and thus the load dispersal capacity. The block shown in Fig. 2(iii) is a further improvement over dentated rectangular block. The block shown in Fig. 2(iv) gives still better interlock and is suitable for fully mechanized paving.

The dentated blocks further can be grouped as shown in Fig. 3 into three categories as under:

- Category A:** Dentated units are designed to key into each other on all four faces and which, by their plan geometry when keyed together, resist the widening of the joint. These blocks are generally capable of being laid in herringbone bond pattern (as explained in Section 8).
- Category B:** These blocks are dentated on only two sides. Their dimensional accuracy of laying helps in bringing about the interlock effect on other faces. Generally, with some exceptions, these blocks can only be laid in stretcher bond, as explained in Section 8.
- Category C:** These are not dentated type but depend on dimensional accuracy for interlocking effect. These blocks can be laid only in a stretcher bond.

The overall dimension of blocks used in various parts of the world ranges as under :

Top surface area : 5,000 to 60,000 mm²

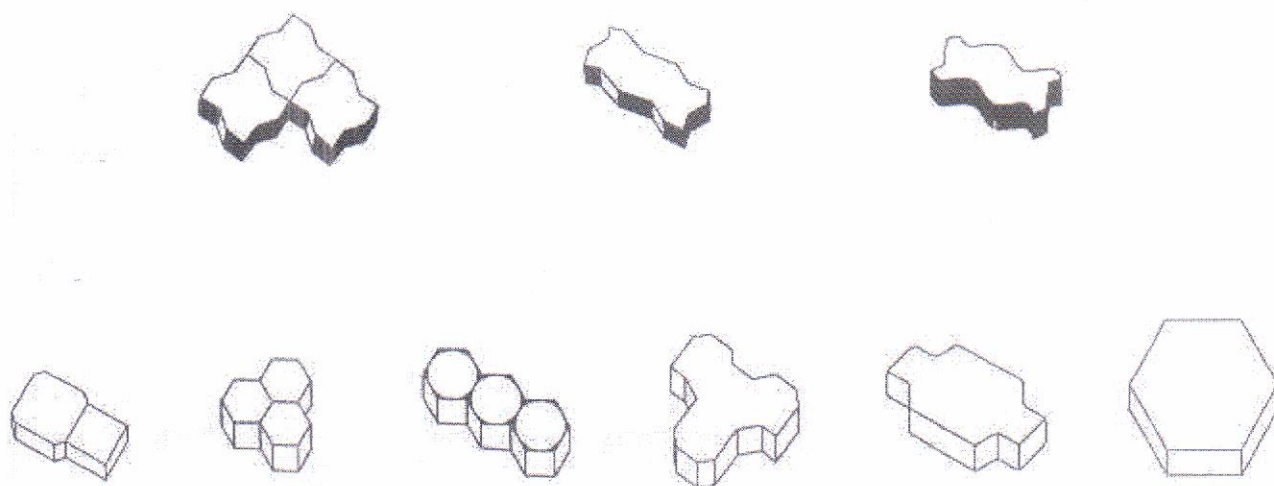
Horizontal dimension not exceeding : 28 cm

$$1 < \frac{\text{Mean length}}{\text{Mean width}} < 3$$

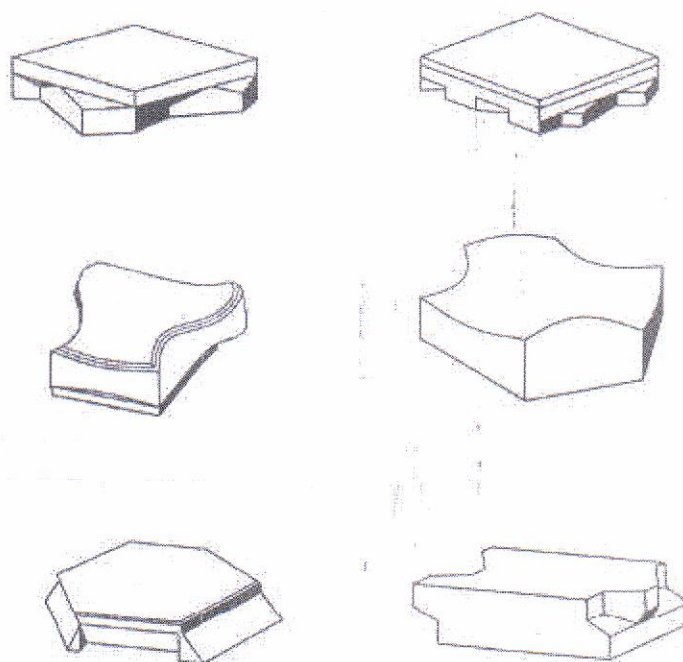
Thickness : Between 60 to 140 mm

Length/Thickness : ≥ 4

In addition to regular blocks described above, supplementary blocks of half size would be required for paving purpose. In the case of rectangular blocks, more number of half blocks would be generally required than other category of blocks.



(i) SOME TYPICAL SHAPES OF INTERLOCKING BLOCKS



(ii) SOME TYPICAL SHAPES OF VERTICALLY INTERLOCKING BLOCKS

Fig. 1. Some shapes of interlocking blocks

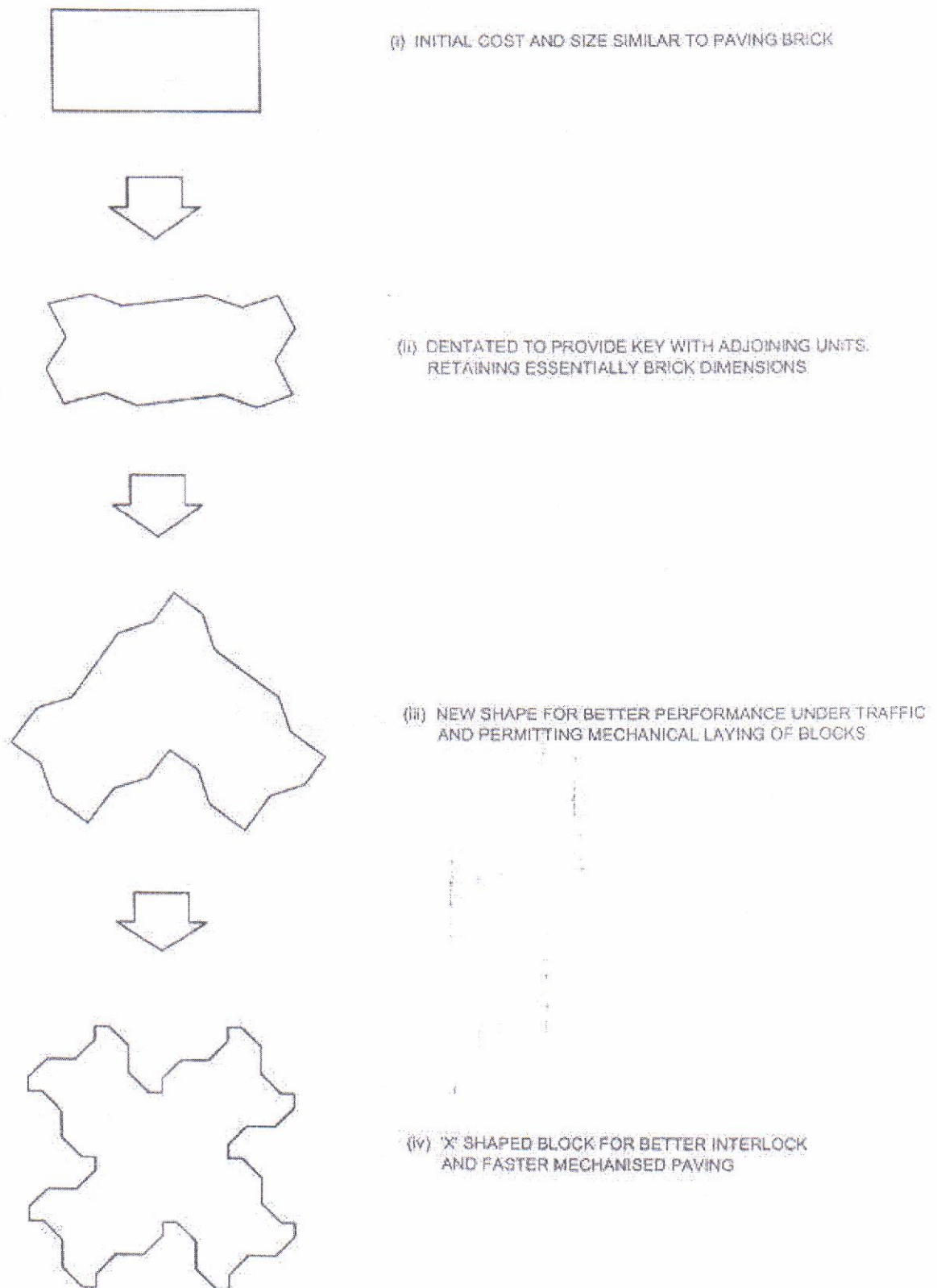


Fig. 2. Basic shapes of blocks











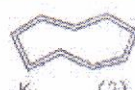


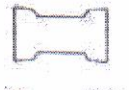

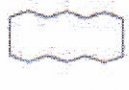

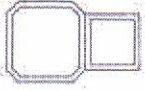




CATEGORY A	 A (1)	 B (1)	 C (1)	 D (1)	 E (1)	 F (1)
CATEGORY B	 G (2)	 H (2)	 I (2)	 J (2)	 K (2)	 L (2)
	 M (2)	 N (2)	 O (2)	 P (2)	 Q (2)	 R (2)
CATEGORY C	 S (2)	 T (2)	 U (2)	 V (2)		
NOTES	(1) SUITABLE FOR A VARIETY OF BONDS INCLUDING HERRINGBONE		(2) SUITABLE ONLY FOR STRETCHER BOND		BLOCKS KNOWN TO HAVE HAD LOAD DISTRIBUTION STUDIES OR TRAFFIC TESTS.	

Fig. 3. Different categories of blocks

Special Grass Blocks

For improving aesthetic looks of paved areas, architects have been making use of block pavement extensively. The numerous paving blocks and their joints mellow down the harshness created by large transverse joints formed in conventional concrete pavement.

For improving aesthetics further, grass blocks have been developed. These when constructed in a grid formation allow space in the pavement for growing grass as shown in Fig. 4. These are best suited for walkways, driveways, etc. Coloured blocks also add to the aesthetic beauty.

4. COMPOSITION OF BLOCK PAVEMENT

4.1. General

Except for the top wearing part of the pavement, the base and sub-base layers are similar to the conventional flexible or rigid pavement. Depending upon the load coming on them, the composition of the pavement differs.

4.2. Typical Pavement Composition

A few typical compositions normally used are given in Figs. 5 and 6.

4.3. Block Thickness

Interlocking concrete blocks come in different thicknesses. These blocks serve as wearing surface but at the same time help in reducing the stresses imposed on subgrade and also help in resisting pavement deformation and elastic deflections similar to the base course of a flexible pavement.

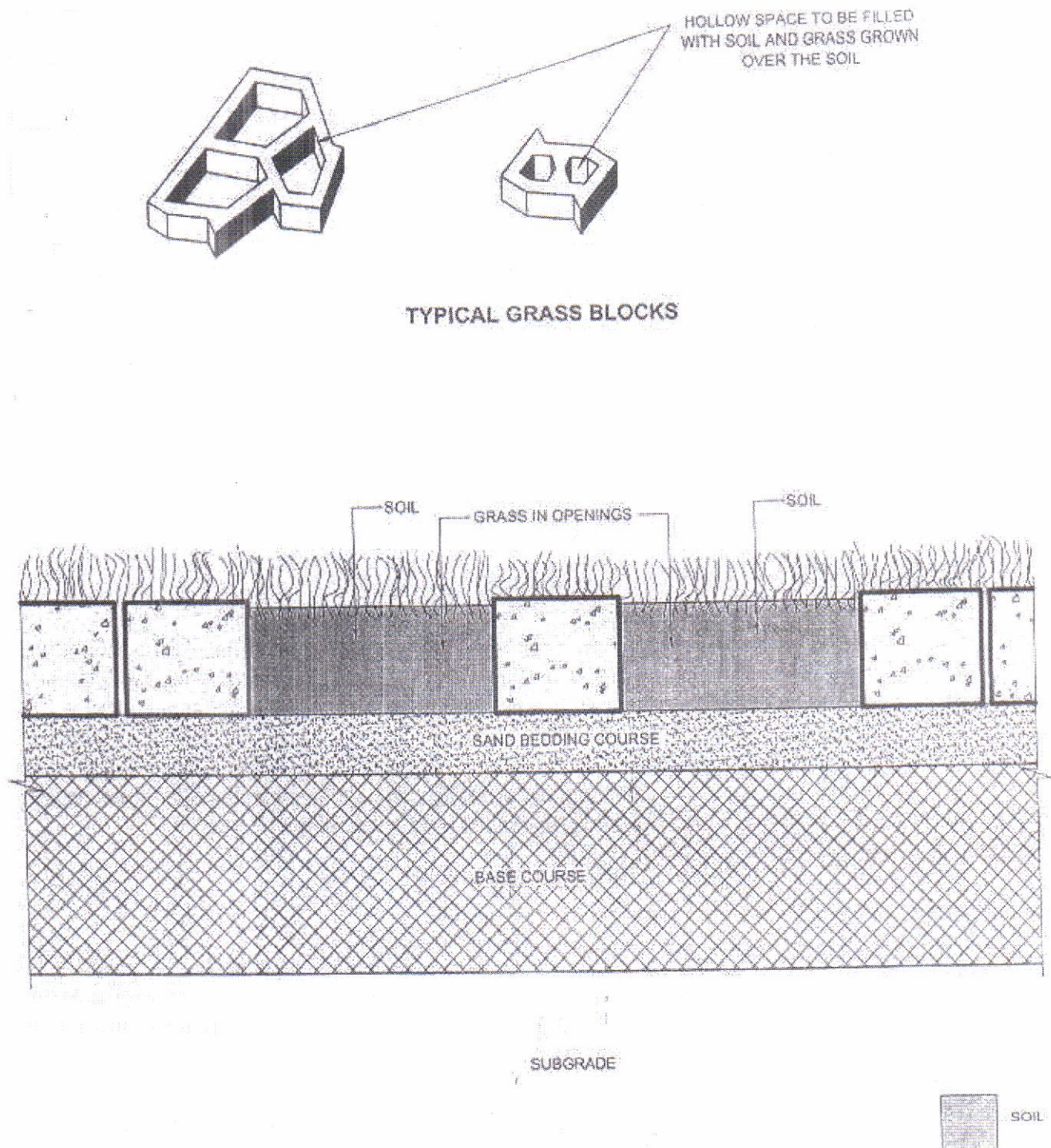


Fig. 4. Grass blocks and the construction technique

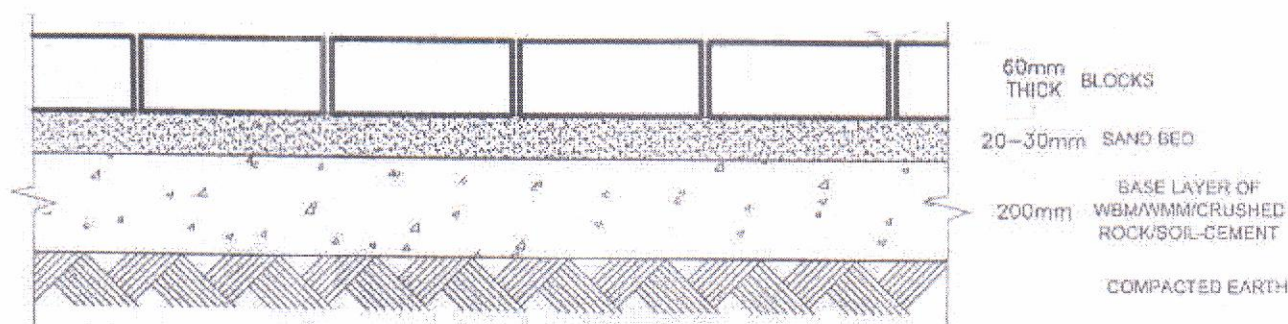


Fig. 5. A Typical cross section of block pavement used in sidewalks/foot-paths/car-parks/cycle track

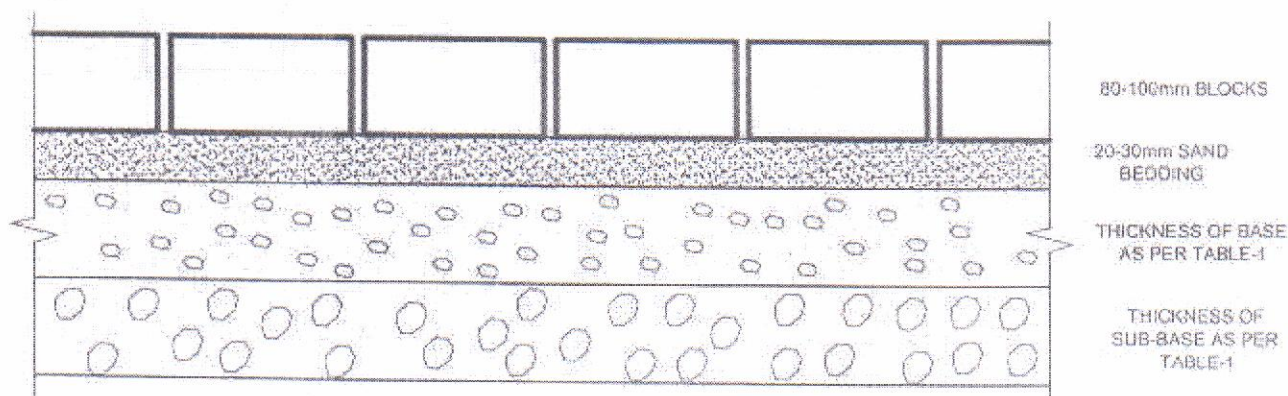


Fig. 6. A typical cross section of block pavement for heavily trafficked roads

For Category 'A' blocks used for light traffic, such as, pedestrians, motor cars, cycles, etc., a block thickness of 60 mm is adequate; for medium traffic, a thickness of 80 mm is generally used; for heavily trafficked roads, Category 'B' blocks of the thickness 100-120 mm are used. Thick blocks are best suited where high volumes of turning movements are involved.

Non-uniformity in thickness of blocks affects the evenness of the surface. A block pavement which is initially paved to a levelled surface will settle unevenly with the movement of vehicles, as shown in Fig. 7. In view of this, all blocks should be of the same thickness, with a maximum allowable tolerance limits of ± 3 mm. Similarly, variations in length and width of blocks should be limited to ± 2 to 3 mm for ensuring uniform joint width and avoiding staggering effect.

4.4. Sand Bedding and Jointing

A layer of sand bedding is provided between block pavement and base/sub-base for the following reasons:

- (i) To provide a cushion between the hard base and the paving blocks
- (ii) The base or sub-base will have some permitted surface unevenness. By providing a layer of sand bed, the paved block can be levelled perfectly.
- (iii) The sand bed acts as a barrier and does not allow propagation of cracks formed in base/sub-base.
- (iv) The sand also helps to keep lower part of the joint filled with sand and provides added interlocking effect.

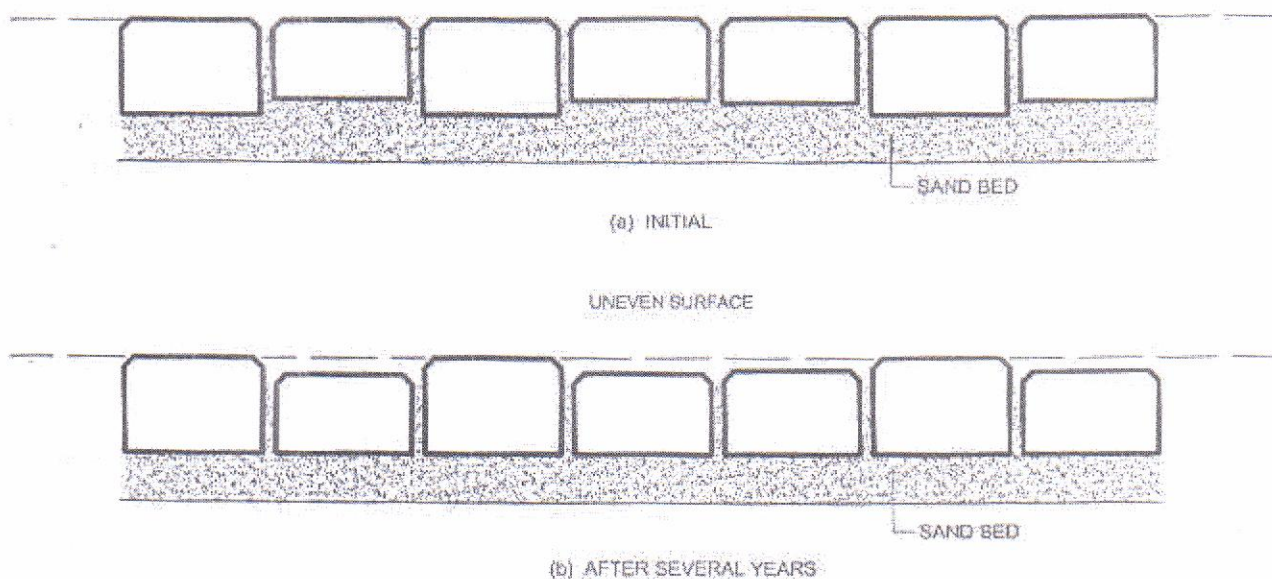


Fig. 7. Effect of thickness variations in paving blocks

The sand bed should not be too thick lest it would be difficult to control the surface level of the blocks. A layer thickness of 20 to 40 mm is found to be satisfactory.

For block pavement to perform satisfactorily, it is necessary that the lower layers are profiled to proper level and finish and that the bedding sand layer is of uniform thickness. Varying thickness of sand bed ultimately results in uneven surface of the pavement.

The grading and quality of sand is very important for the block pavement to perform satisfactorily. The sand used should be free from plastic clay and should be angular type. It should not be of degradable type for e.g., sand produced from lime stone, etc. is likely to get powdered under the loading.

Joints between blocks are filled by fine sand. Normally, the bottom 20 to 30 mm of the joint gets filled with bedding sand, whereas, the remainder space has to be filled with jointing sand by brooming it from the top. The joints are normally 2 to 4 mm wide.

4.5. Base and Sub-base Layers

These layers are the important structural layers of a block pavement. The materials used for base construction consist of either bound material like lean concrete or soil-cement or bituminous layers or unbound materials like wet mix macadam or WBM. The sub-bases are generally of granular material. The sub-base can function as drainage layer as well, provided proper disposal arrangement for water is made. The base course layer is normally provided where heavy vehicular traffic is likely.

Besides intensity of loading, the type of soil encountered determines the type and thickness of base and sub-base. For weak subgrade soils like clays, where ground water table is shallow, bound bases are preferred.

4.6. Edge Restraint Blocks and Kerbs

Concrete blocks on trafficked pavements tend to move sideways and forward due to braking and manoeuvring of vehicles. The tendency to move sideways has to be counteracted at the edges by special edge blocks and kerbs. The edge blocks should be designed such that the rotation or displacement of blocks is resisted. These are to be made of concrete of high strength to withstand the traffic wheel loading without getting damaged. These members should be manufactured or constructed in-situ to have at least a 28-day compressive strength of 30 MPa or flexural strength of 3.8 MPa. As far as possible the edge blocks should have vertical face towards the inside blocks. A few typical edge blocks are also shown in Fig. 8.

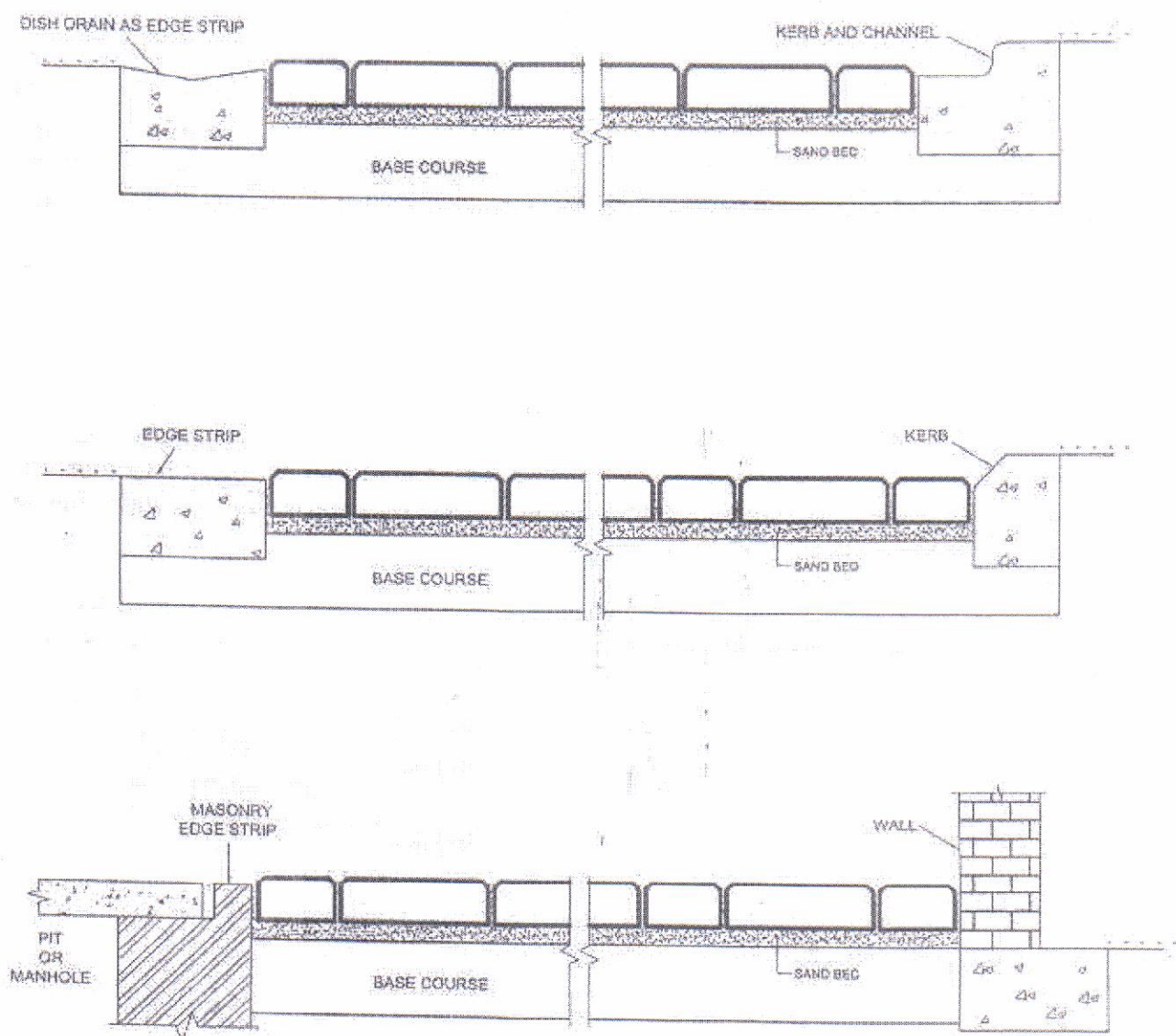


Fig. 8. Edge restraints.

The road kerbs provided on the edge of the road also serve the purpose of edge blocks as shown in Fig. 8. In case the kerbs are not provided, it has to be replaced by edge strips.

5. STRUCTURAL DESIGN OF CONCRETE BLOCK PAVEMENT

5.1. Suggested Design Procedure

Design procedures have been developed by agencies abroad based on successful performance, or mechanistic principles. They cover a variety of roads ranging from the lightly trafficked to the heavily trafficked. In the absence of research in India, it is recommended that the catalogue of designs given subsequently may be used.

5.2. Lightly Trafficked Pavements

Pedestrian side-walks, footpaths, cycle tracks, car parks and malls are lightly trafficked. In such situations, the pavement can consist of blocks 60 mm thick laid over a sand bedding 20-30 mm and a base course 200 mm thick. The base course can be in WBM/WMM/crushed stone/soil-cement. This design can be adopted for the range of subgrade soils met with in India. A typical cross-section is given in Fig. 5.

5.3. Block Pavements Subjected to Commercial Traffic

City streets and highway sections subjected to commercial traffic (trucks and buses) require a heavier section. Though design methods based on empirical approach and mechanistic behaviour are available, enough work has not been done in India to evolve the country's own design procedure. In the absence of such knowledge, the ad-hoc design catalogues based on international experience as given in Table 1 are suggested for adoption. A design life of 20 years can be considered for determining the repetitions of standard axles.

5.4. For block pavements for industrial applications like container yard and port wharf and roads and warehouses the following thickness is recommended, based on international experience:

Block	:	100 mm
Sand Bedding	:	30-50 mm
Hydraulically bound base	:	300 mm
Granular sub-base (out of which the bottom 150 mm is a drainage layer)	:	300 mm

6. MATERIALS

6.1. General

The quality of materials, cement concrete strength, durability and dimensional tolerances, etc. are of great importance for the satisfactory performance of block pavements. These aspects and the

TABLE 1 : DESIGN CATALOGUE FOR PAVEMENT THICKNESS

Traffic and Road Type	Subgrade CBR (%)		
		Above 10	5-10
• Cycle Tracks, Pedestrian Footpaths	Blocks Sand Bed Base	60 20-30 200	60 20-30 200
• Commercial Traffic Axle Load Repetitions less than 10 msa • Residential Streets	Blocks Sand Bed WBM/WMM Base Granular Sub-base	60-80 20-40 250 200	60-80 20-40 250 250
• Commercial traffic Axle Load Repetitions 10-20 msa • Collector Streets, Industrial Streets, Bus and Truck Parking Areas	Blocks Sand Bed WBM/WMM Base Granular Sub-base	80-100 20-40 250 200	80-100 20-40 250 250
• Commercial traffic Axle Load Repetitions 20-50 msa • Arterial Streets	Blocks Sand Bed WBM/WMM Base or WBM/WMM Base and DLC over it* Granular Sub-base	80-100 20-40 250 150 75 200	80-100 20-40 250 150 75 250

- Notes :
1. Thickness of layers given above are in mm.
 2. Granular sub-base should have at least 150 mm layer at the bottom which is drainable.
 3. A typical cross-section is given in Fig. 6.
 4. If the subgrade soil has a CBR of less than 5, it should be improved by suitable stabilisation technique to bring the CBR value to 5.
 5. msa denotes repetitions in million standard axles
- * in case of roads having inadequate drainage or heavy rainfall areas (above 1500 mm per annum)

block manufacturing process itself, which immensely influences the quality of paving blocks, are broadly outlined in the subsequent paragraphs. The desired engineering properties of bedding/jointing sand layer beneath the block, the base course and sub-base materials are also described.

6.2. Salient Mix Design Aspects

The commonly used processes for the manufacture of pre-cast cement concrete paving units require dry, low-slump mixes. The desired characteristics of the mix are as under:

Water/cement ratio	:	0.34 to 0.38
Water content of the mix	:	5 to 7% of total mix
Quantity of cement in mix	:	Generally not less than 380 kg/m ³ depending on the equipment being used for block making. Upper limit of cement shall not be more than 425 kg/m ³ . Flyash also can be used in the mix, replacing Ordinary Portland Cement to an extent of 35 per cent.

The above values are for general guidance only. The actual mix design has to be made to suit each individual requirement.

Aggregate/cement ratio	:	3:1 to 6:1
Aggregates	:	Should be sound and free from soft or honeycombed pieces. The proportion of coarse aggregate in the mix is typically 40 per cent and the fine aggregate (sand) 60 per cent. The size of coarse aggregate should lie between 6 mm and 12 mm and the gradation should be in the recommended range for cement concrete mixes in general.
Strength	:	In general terms, the paving block must have adequate strength to withstand handling, construction stresses and effects of traffic, though the strength as such is not considered a vital factor in the satisfactory performance of a block pavement. However, it is suggested that the minimum compressive strength of a single block should be above 30 MPa.
Addition of Pigments	:	To provide the desired colour to paving blocks, appropriate type and amount of pigments are added during mixing, in the form of powder or slurry. Although organic pigments render brighter colours than inorganic pigments, the former are adversely affected by the alkaline environment of concrete and do deteriorate with time. Inorganic pigments, mostly metal oxides, are more durable and hence preferred for consistency and purity. Saturation of colour takes place with a pigment volume of around 5 to 9 per cent of cement content. Pigments should be finer than cement (fineness value between 2 and 15 m ² /gm). For the same slump, addition of pigments

Other Additives

requires increase in mixing water, which in some cases may lead to lowering of flexural and compressive strength of concrete; therefore, suitable adjustments in mix proportions may become necessary.

Under special circumstances, super-plasticizers at around 0.4 per cent of cement by weight may be added for high early strength. Water repellant admixtures of calcium stearate are sometimes used to reduce water absorption. Air entraining agents, when added to the mix, cause some reduction in the needed amount of cement. Further reduction is achieved by substituting part of the cement with blast-furnace slag or pozzolanas like flyash; besides reducing cost, these also control "efflorescence" (surface deposition of salts as a result of water movement upwards).

6.3. Manufacture of Paving Blocks

The method of manufacture of paving blocks has an important bearing on the quality, durability and level of finish - dimensional tolerance, etc. all of which reflect on the ultimate performance of the block pavement during service. At the very outset, therefore, it is to be emphasized that hand-casted concrete blocks are unacceptable for use and that an appropriate plant should be used which would make it possible to apply high pressure together with controlled vibration. Adaptation of production facilities designed for high quality hollow masonry blocks, though feasible, is not as economical and as efficient as the use of purpose designed machinery for paving block manufacture. Essentially, the manufacturing process involves compacting concrete, in a steel mould clamped to a vibrating table, by hydraulic pressure.

Concrete is fed into the mould from a hopper by a drawer - if a second hopper is added, a block can be made of two kinds of concrete having "backing" and "facing" surfaces. In the "facing" of the block, the top 5 mm has greater amounts of cement and sand to make it more durable and skid-resistant, and extra pigment is added for the coloured face vis-a-vis the rest of the block. In the first stage of compaction, pre-vibration is effected by running the vibrators attached to the vibratory table, the frequency generally being in the range of 50 to 100 Hz. In the second stage of compaction, compression pressure is applied to the tamper heads, also fitted with vibrators for a high level of surface finish. Blocks are extruded from mould by forcing down the tamper heads, after the vibrating table is disengaged from the mould. The blocks thus prepared are stacked either in a single layer or multiple layers for curing, depending on the plant used being single layer or multi-layer.

6.4. Dimensional and other Requirements of Paving Blocks

For normal paving work, the length of a paving block should ordinarily be not greater than twice the mean width; the thickness is a minimum of 60 mm; the maximum length generally not exceeding 280 mm; the width generally is in the range 75 to 140 mm with a maximum chamfer of 10 mm (preferably chamfer should be in the range 3-5 mm). The sides of the block should be perpendicular

to the top and bottom faces except that the top edge may be chamfered. The blocks should have the following dimensional tolerances:

Plan dimensions	±	2 mm
Thickness	±	3 mm

To ensure durability, the average water absorption in a block should not exceed 5 per cent; and for cold regions in a standard freeze-thaw durability test, the weight loss should not exceed 1 per cent.

In situations, where parts of blocks are to be used e.g., around manholes, the block should be purpose-cut at site.

It is to be recognized that variations in the thickness of blocks used for a paving job can be a major cause for the loss of surface profile, as shown in Fig. 7 on an exaggerated scale. In the interest of maintaining a good surface profile, the block thickness should be controlled carefully; this can be done to advantage by adopting multi-layer method of manufacture of paving blocks, which reduces the variation in block thickness.

Annexure give suggested technical specifications for laying block pavements.

6.5. Bedding and Joint Filling Sand

6.5.1. **Bedding sand :** It is well established that if proper attention is not paid to the quality of bedding sand, and if the thickness of the bedding sand layer is not uniform enough, serious irregularities in surface profile can result; excessive differential deformation and rutting can occur early in service life of the block pavement. The desired gradation of bedding sand should be as under :

IS Sieve Size	Percent Passing
9.52 mm	100
4.75 mm	95-100
2.36 mm	80-100
1.18 mm	50-95
600 micron	25-60
300 micron	10-30
150 micron	0-15
75 micron	0-10

Care should be taken to see that single-sized or gap graded sands or sands with excessive amount of fines or plastic fines should not be used. The shape of sand particles should preferably be sharp rather than rounded, since the sharp sands possess higher strength and resist the migration of sand from under the block to less frequently trafficked areas. Even though sharp sands are relatively more difficult to compact than rounded sands, the use of sharp sands should be preferred for the more heavily trafficked pavements. The bedding sand should be free of deleterious materials.

6.5.2. Joint filling sand : The gaps in between two paving blocks (typically about 3 mm wide) need to be filled by sand, relatively finer than the bedding sand. The desired gradation for the joint filling sand is as under:

IS Sieve Size	Per cent Passing
2.36 mm	100
1.18 mm	90-100
600 micron	60-90
300 micron	30-60
150 micron	15-30
75 micron	0-10

It is necessary to restrict the fines (silt and/or clay) to 10 per cent, since excessive fines make joint filling very difficult. Similarly, it is not advisable to use cement in the joint filling sand which will not only make it difficult to completely fill the joints but would also adversely affect the desired flexibility characteristics of the paving block layer. The joint filling sand should be as dry as possible; otherwise complete filling of joints will be difficult. To overcome the problem of efflorescence on the surface of paving block layer, the joint filling sand should be washed to remove soluble salts.

6.6. Base Material

The engineering properties of base materials, which include load spreading properties to reduce stresses on the subgrade and desired drainage characteristics, have an important bearing on the performance of a block pavement. Although, local availability and economics generally dictate the choice of base material at the design stage, the commonly used materials considered suitable for base courses are unbound crushed rock, water-bound macadam, wet mix macadam, cement bound crushed rock/ granular materials, and lean cement concrete.

In broad terms, wherever the subgrade is weak (having a CBR value below 5) use of bound granular materials, like, cement treated crushed rock, requiring a relatively thinner base, should be preferred while for high strength subgrades, unbound crushed rock can be used. The climatic and environmental factors also need to be considered during the choice of a base material.

6.7. Sub-base Material

Generally, a sub-base is warranted where commercial traffic is expected. The quality of sub-base materials is inferior to the base materials and includes natural gravels, cement treated gravels and sands and stabilized subgrade materials. The quality of sub-base materials should be in conformance with IRC:37-2001.

7. DRAINAGE

Block pavement with joints filled with sand is not a waterproof layer and hence care has to be taken to drain out the surface water seeping through the joints in initial stage of the construction. This

water can find way to sand bed below, base, sub-base and subgrade layers. Unless these layers are free draining, appropriate drainage arrangement has to be provided. The drainage provided generally consists of subsurface drains surrounded by filter material or a geotextile, which would allow the water to pass through and at the same time prevent the escape of bedding/jointing sand. Typical subsurface drainage arrangement used in block pavement is shown in Figs. 9 and 10.

Shown in Fig. 11 is a drainage system with no fines concrete provided below the sand bed. The water collected is to be taken through 80 mm diameter perforated pipe.

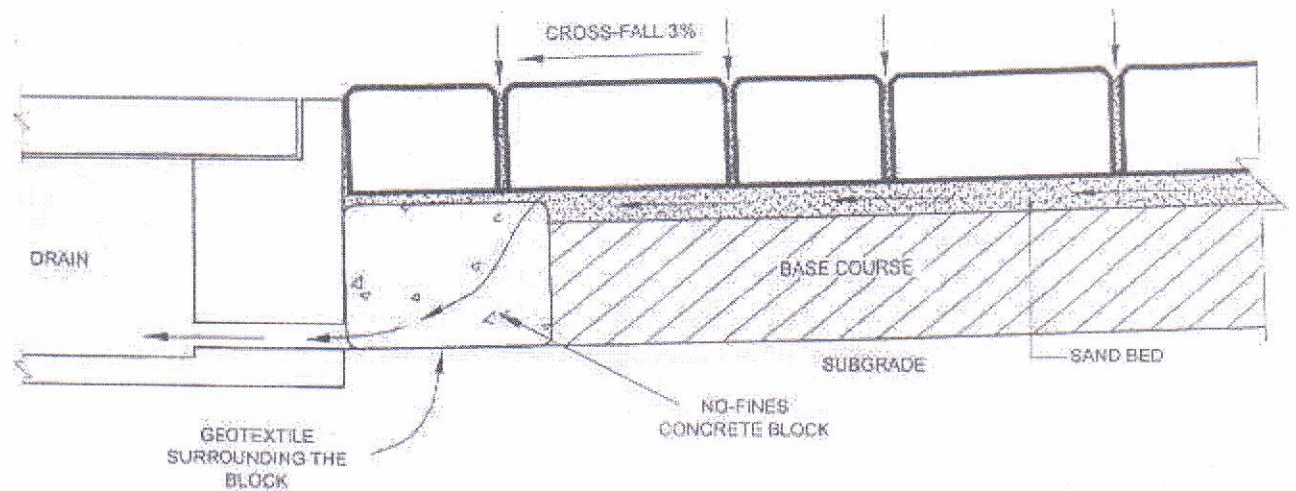


Fig. 9. Surface drainage in a block pavement

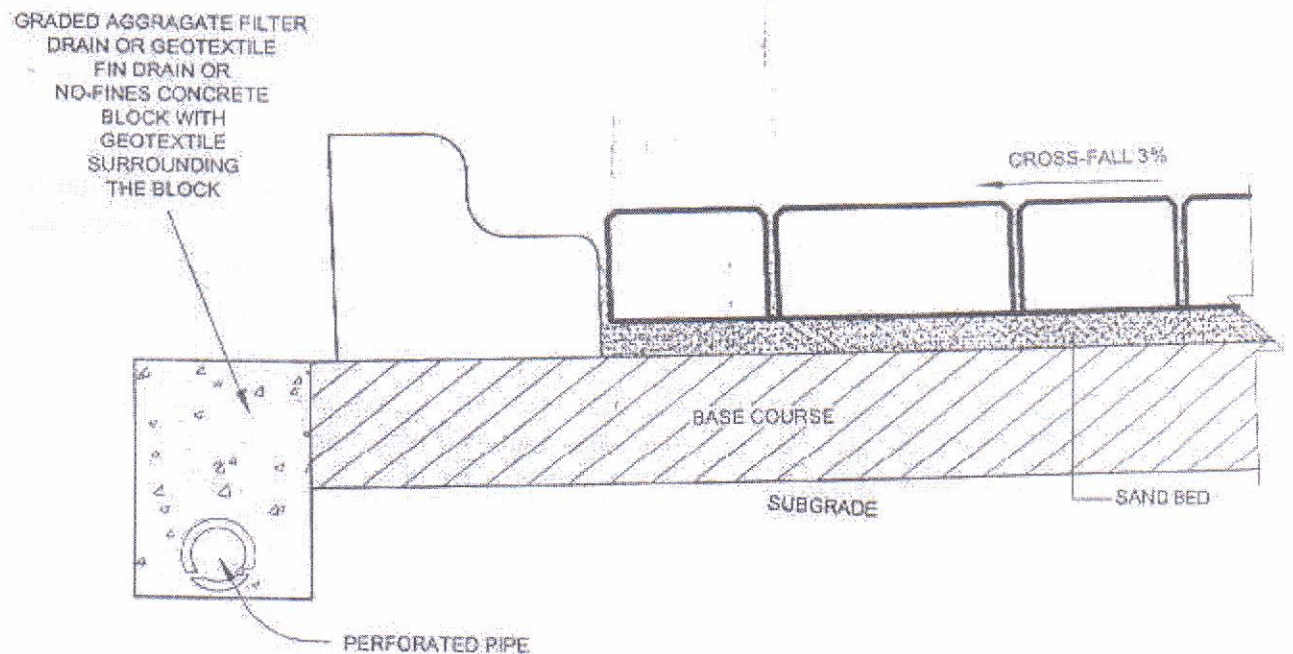


Fig. 10. Surface drainage in a block pavement

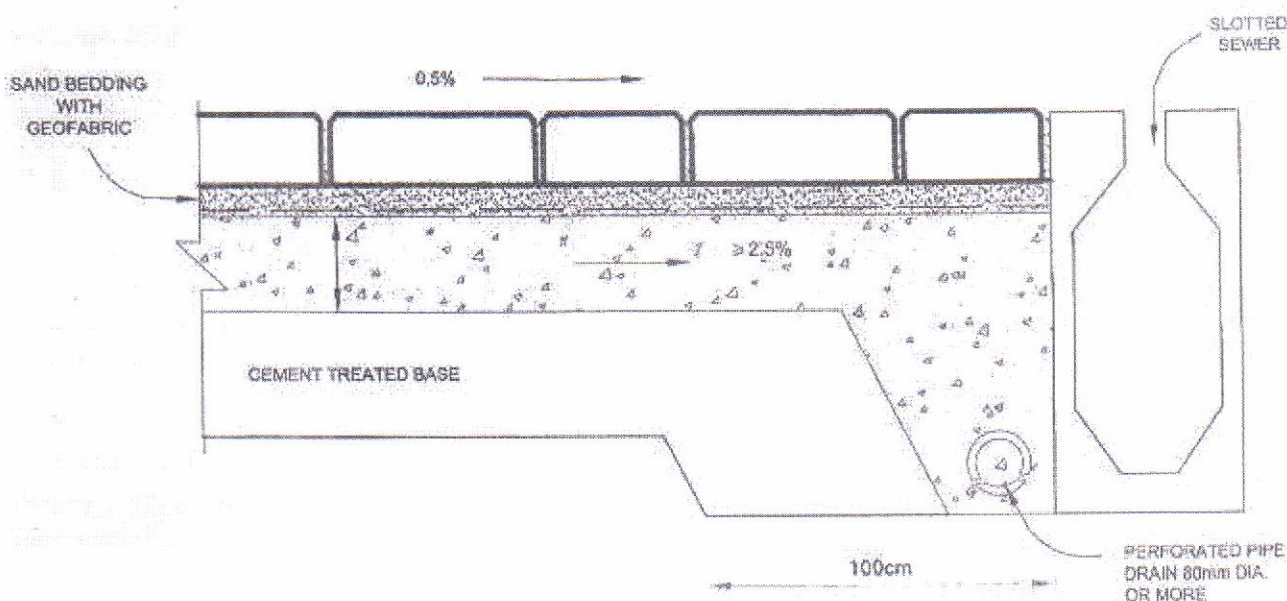


Fig. 11. Heavy trafficked concrete block pavement structure with a base course of no-fines concrete for drainage

A crossfall of 2 per cent slope is generally sufficient to drain the surface run-off but it is desirable to provide 3 per cent crossfall in the case of heavily trafficked roads to avoid formation of water puddles. The block pavement should be at least 5 mm above the manholes, side drains, etc.

8. CONSTRUCTION

8.1. General

The construction of block pavement involves preparation of subgrade, sub-base and base course layers, bedding sand and finally the laying of blocks. The block paving can be done entirely by manual labour. However, for efficient construction work, the work force has to be properly trained for this specialised job. Paving can also be done by mechanical means.

8.2. Preparation of Subgrade

This is the foundation layer on which the block pavement is constructed. Like in conventional pavements the water table should be at a minimum depth of 600 mm below the subgrade. Subgrade should be compacted in layers of 150 or 100 mm thickness as per IRC:36-1970. The prepared subgrade should be graded and trimmed to a tolerance of ± 20 mm of the design levels, and its surface evenness should have a tolerance of within 15 mm under a 3 m straight edge.

8.3. Base and Sub-base Course

Base and sub-base courses are constructed in accordance with standard procedures contained in the relevant IRC Specifications, like, IRC:37-2001, IRC:50-1973, IRC:51-1993, IRC:63-1976,

IRC:19-1977. When cement bound base are proposed it may be constructed using rolled lean concrete as per IRC:SP-49. The quality control specified in IRC:SP-11 shall apply. Constructing the layers to proper level and grade is very essential to maintain the level and surface regularity of the block pavement.

8.4. Placing and Screeding of Bedding Sand

The thickness of the sand bed after compaction should be in the range of 20-40 mm, whereas, in the loose form it can be 25 to 50 mm. It is preferable to restrict the compacted thickness to 20-25 mm to reduce the risk of any localized precompaction, which would affect the final block surface level. Bedding sand should not be used to fill-up local depressions on the surface of a base or sub-base. The depressions should be repaired in advance before placing sand.

Sand to be used should be uniformly in loose condition and should have a uniform moisture content. Best moisture content is that when sand is neither too wet nor too dry and have a value of 6 to 8 per cent. Requirement of sand for a day's work should be prepared and stored in advance and covered with tarpaulin or polythene sheets.

The processed sand is spread with the help of screed boards to the required thickness. The screed boards are provided with nails at 2-3 m apart which when dragged gives the desired thickness. The length of nail should take into account the surcharge to be provided in the uncompacted thickness. Alternatively, the screed can be dragged on edge strips kept on both sides as guide. Asphalt paver can be employed in large projects. The sand is subsequently compacted with plate vibrators weighing 0.6 tonnes or more. Level checks shall be carried out on a grid pattern to establish that the desired level is achieved. Local correction can be done either by removing or adding extra sand followed by levelling and compacting the layer. There will be some settlement of sand after the blocks are placed and compacted, which must be allowed for, while fixing the level of sand bed.

The effect of undulating surface of base or sub-base on the profile of block pavement is explained in Fig. 12. The blocks will settle after trafficking in such a manner that the surface profile becomes parallel to base/sub-base profile. Sand bed assumes uniform thickness under moving loads.

8.5. Laying of Blocks

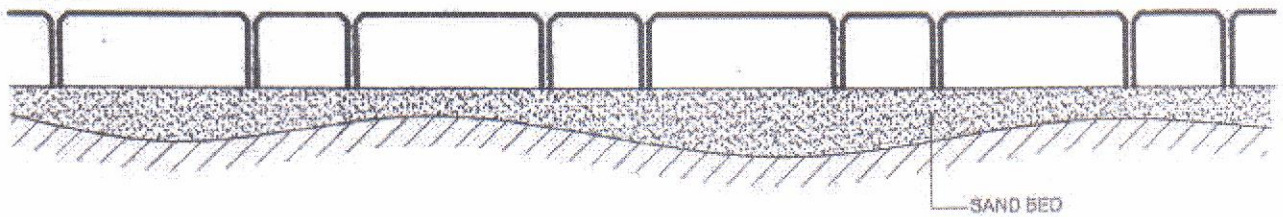
Blocks can be laid generally by manual labour but mechanical aids like hand-pushed trolleys can expedite the work.

Normally, laying should commence from the edge strip and proceed towards the inner side. When dentated blocks are used, the laying done at two fronts will create problem for matching joints in the middle. Hence, as far as possible, laying should proceed in one direction only, along the entire width of the area to be paved.

While locating the starting line, the following should be considered:

- On a sloping site, start from the lowest point and proceed uphill on a continuous basis, to avoid down hill creep in incomplete areas.
- In case of irregular shaped edge restraints or strip, it is better to start from straight string line as shown in Fig. 13.
- Influence of alignment of edge restraints on achieving and maintaining laying bond.

(a) BEFORE COMPACTION OF BEDDING SAND, BASE COURSE SURFACE IS OUT OF LEVEL TOLERANCE



(b) AFTER COMPACTION OF BEDDING SAND, BLOCK SURFACE WILL BE OUT OF SHAPE

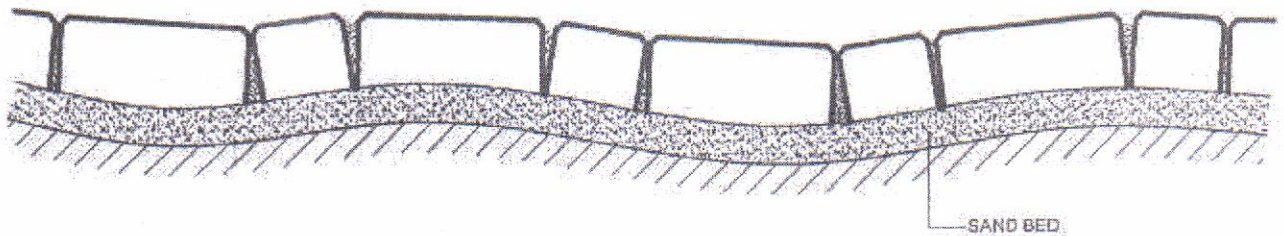


Fig. 12. Effect of base-course surface shape on bedding sand and block surface shape

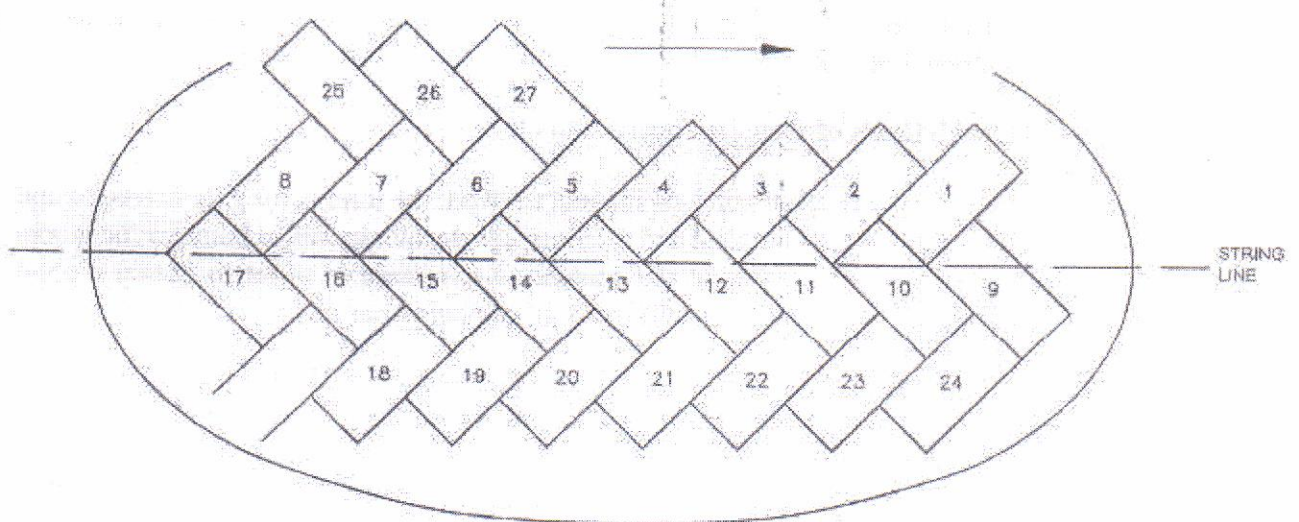


Fig. 13. Starting at irregular shaped edge restraint

8.6. Bonds or Patterns of Laying Blocks

The blocks can be placed to different bonds or patterns depending upon requirement. Some popular bonds commonly adopted for block paving are:

- (i) Stretcher or running bond
- (ii) Herringbone bond
- (iii) Basket weave or parquet bond

The typical layout of these bonds are given in Fig. 14.

8.7. Establishing the Laying Pattern

In relation to the starting line, the blocks should be placed at the correct angle to achieve the final orientation as required by the laying pattern. If the edge restraint is straight and suitably oriented, the first row of blocks can abut it. For irregular-shaped and unfavourably oriented edge restraints, a stringline should be established a few rows away to position the first row.

With the help of gauges, the joint width specification (2 to 4 mm) should be checked in the first few square metres, where it should be ensured that the block alignment is correct. The laying patterns and face should be established (Fig. 15) to permit fast and easy laying without the necessity of forcing a block between previously positioned blocks. To start with, full blocks should be used; only subsequently, cutting and in-filling at edges be permitted. Under no circumstances should the blocks be forced or hammered into the bedding sand at this stage of laying. For cutting paving blocks, hydraulic or mechanical block cutters, or power saws are used. Cut units less than 50 mm minimum dimension should not be used, as these are difficult to cut accurately and can be dislodged under traffic. Where space does not permit use of a larger segment, use premixed concrete or a sand-cement mortar instead.

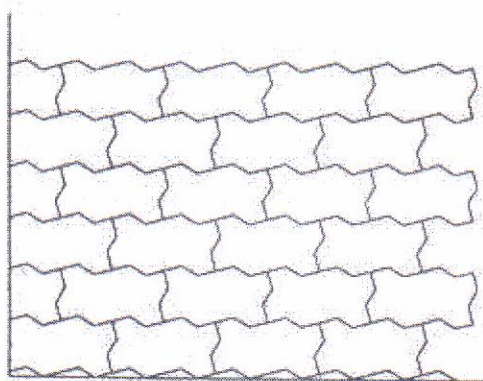
The control over alignment, laying pattern and joint widths can be maintained by the use of chalked string lines, at about 5 m intervals.

8.8. Different Methods of Construction of Block Pavement

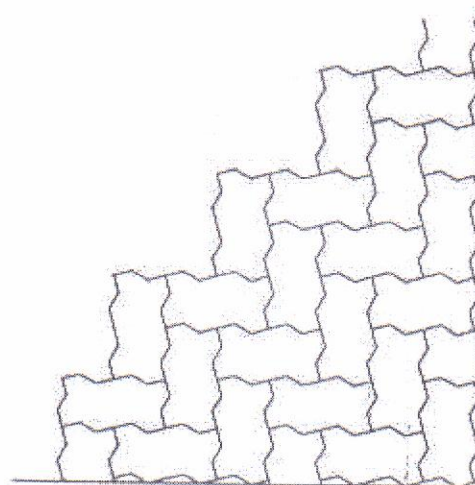
8.8.1. Manual methods : In the traditional manual method, the sand is roughly screeded and a skilled worker (called a pavior) levels the sand and then embeds the block using a hammer; he works backwards so as to have a continuous view of the completed pavement in order to obtain a good finish. A pavior, along with an assistant, can lay 50 to 75 m² of paving per day.

An alternative to the above method, the block layers (generally unskilled labourers) work on the completed surface, moving forward.

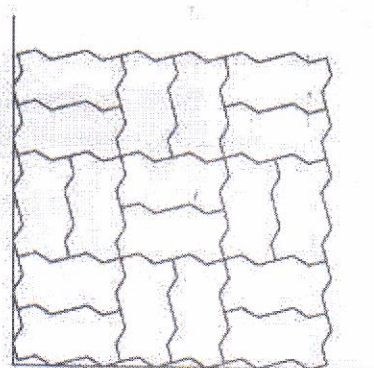
For optimum output, it is advantageous to select an easy fitting block shape, with the desirable size being that which can be easily accommodated in the worker's hand; in addition, the blocks should be chamfered for easy handling and their weight should preferably be less than 4 kg.



(a). STRETCHER BOND OR RUNNING BOND



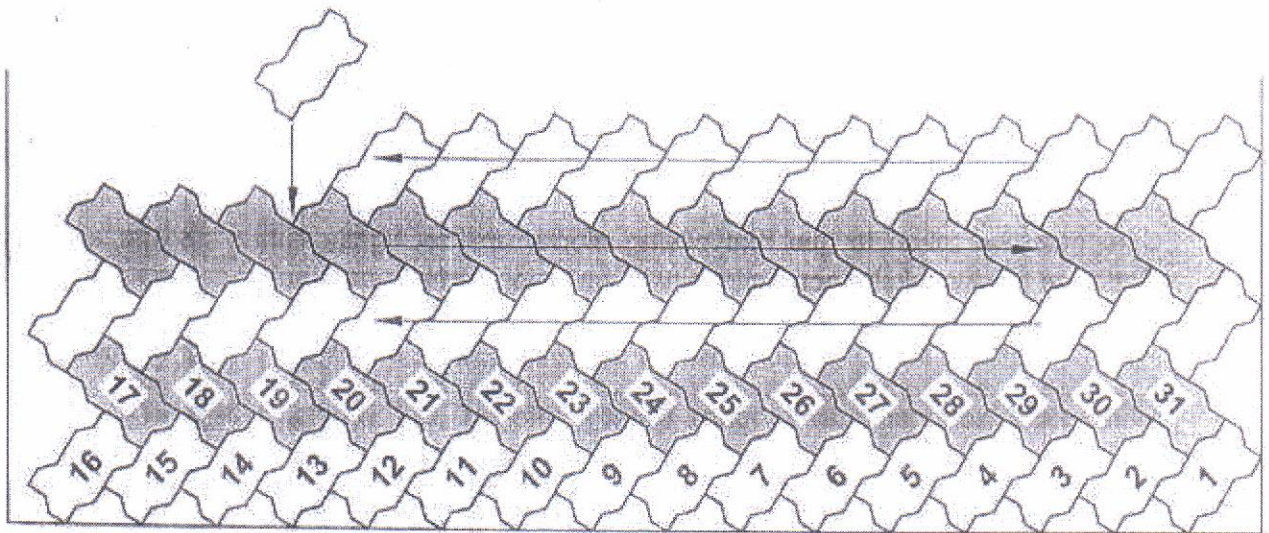
(b). HERRINGBONE BOND



(c). BASKET WEAVE OR PARQUET BOND

Fig. 14. Typical bond or laying pattern of bond

(A) AXIS OF UNITS NOT PARALLEL TO EDGE RESTRAINTS
-ONLY ONE OPERATOR CAN WORK ON THE LAYING FACE



(B) AXIS OF UNITS PARALLEL TO EDGE RESTRAINTS -MORE
THAN ONE OPERATOR MAY WORK ON THE LAYING FACE

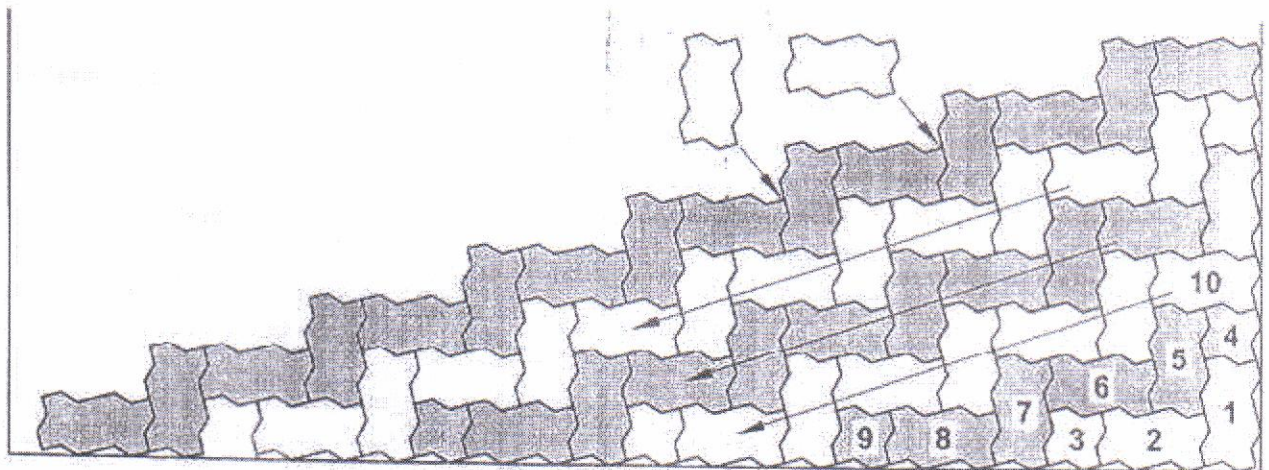


Fig. 15. Establishing laying face for blocks in herringbone bond

The output of finished pavement varies widely with training of workmen, over a wide range from a low of 20 to a high of 120 m²/man-day; the higher outputs being for industrial hard standings where intrusion like manholes, etc. are minimal. To keep up the speed of work, it is important to maintain an adequate supply of paving blocks to the laying site for manual paving. Ordinarily, hand pushed trolleys are adequate for the purpose, but for large projects employing a number of laying teams, use of powered trolleys is preferable.

Care must be taken to see that paving blocks are not tightly butted against each other, otherwise there could be non-uniformity in the laying patterns and the blocks may spall or even crack. Joint widths of 2 to 4 mm can be maintained if, when laying a paving unit, it is held lightly against the face of an adjacent laid unit and allowed to vertically slide into position.

Since each workman may produce slightly different joint widths, it is desirable to rotate workmen along the workface, and also periodically interchange the personnel laying and transporting blocks.

The average joint width can be measured and checked, by determining statistically the representative values of average length and breadth of blocks at the project site and then obtaining average distance between joints, say 40 blocks apart; or it can be done by measuring joint widths directly, using a calibrated, hardened steel mandrel which is forced into joints at a series of randomly selected locations, to obtain a statistically representative figure.

8.8.2. Mechanised methods : Mechanised laying requires the use of specialised equipment for transporting and placing clusters of paving blocks. The size of paving block cluster suitable for paving, is usually 0.3 to 0.5 m² in area for hand-operated equipment; for fully mechanised equipment, the cluster surface area can be upto about 1.2 m². These clusters are designed to maintain a joint space of about 3 mm between blocks, when clamped together (Fig. 16).

Since the blocks are placed in separate clusters, there exists the possibility of damage if joints between adjacent clusters run uninterrupted throughout the pavement. To overcome this problem, clusters may be arranged so that the joints are periodically staggered both along and across the cluster axis or link blocks are installed by hand across these joints. (Fig. 17).

Mechanised laying must be coordinated with the manufacturer, so that the blocks are delivered stacked on pallets in the required pattern; in some cases, spacing ribs may be cast on the sides of blocks to preserve the required joint spacings.

8.8.3. Compaction : For compaction of the bedding sand and the blocks laid over it, vibratory plate compactors are used over the laid paving units; at least two passes of the vibratory plate compactor are needed. Such vibratory compaction should be continued till the top of each paving block is level with its adjacent blocks. It is not good practice to leave compaction till end of the day, as some blocks may move under construction traffic, resulting in the widening of joints and corner contact of blocks, which may cause spalling or cracking of blocks. There should be minimal delay in compaction after laying of the paving blocks to achieve uniformity of compaction and retention of the pattern of laying; however, compaction should not proceed closer than 1 m from the laying face, except after completion of the pavement.

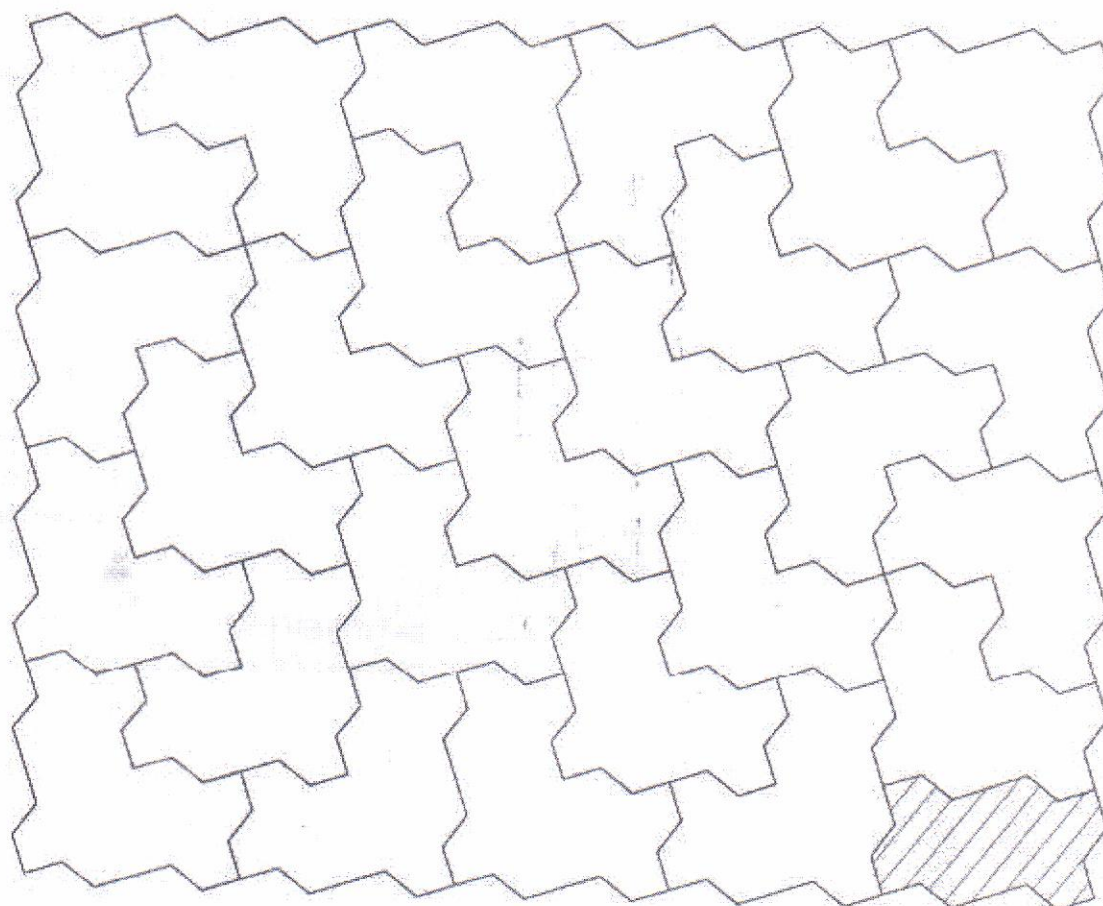
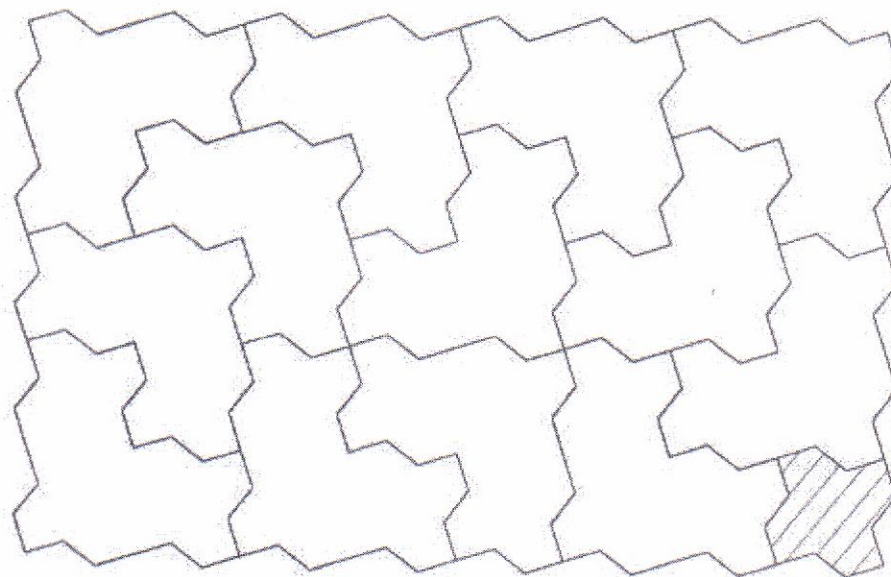


Fig. 16. Typical block cluster in mechanised laying

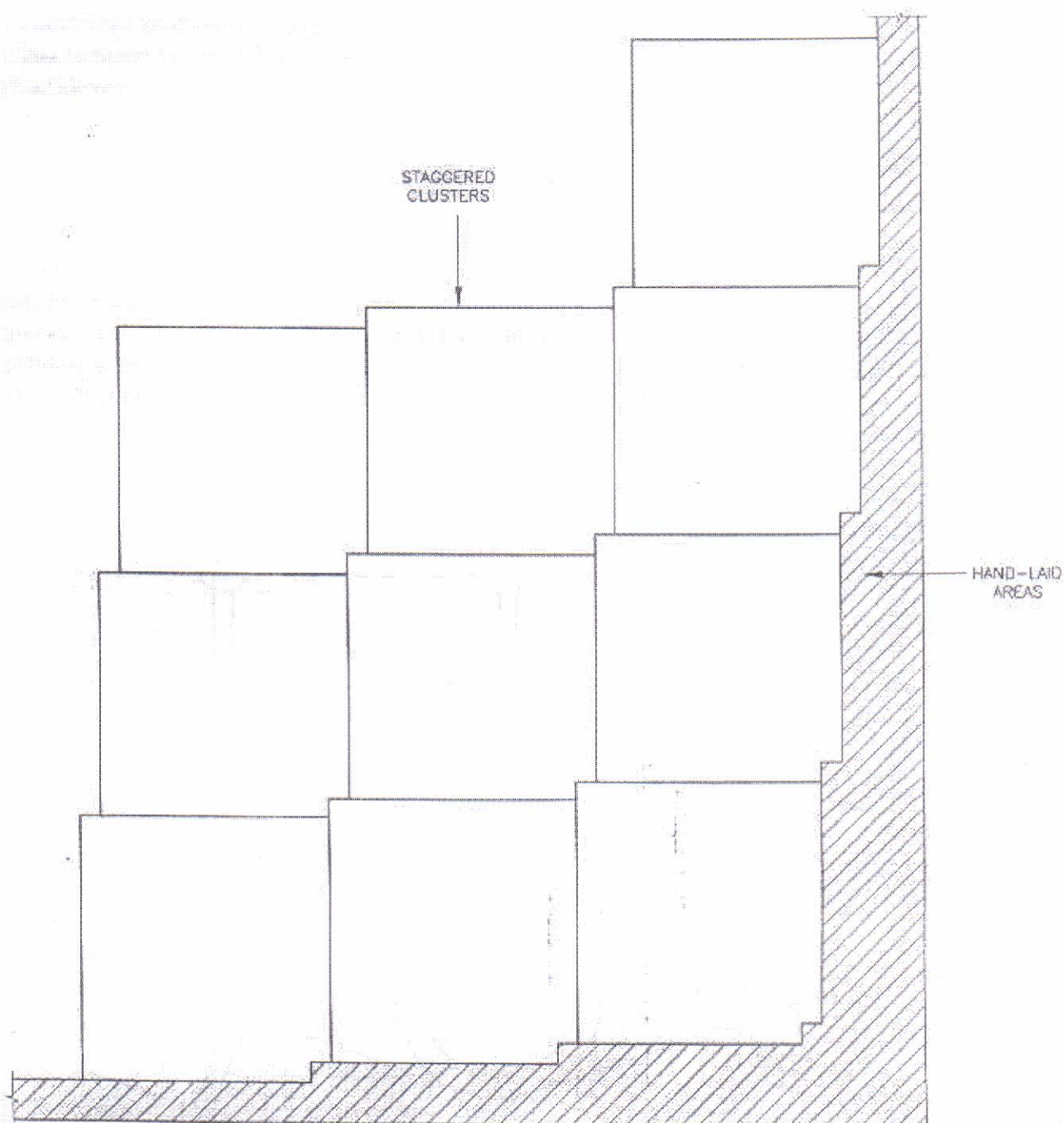


Fig. 17. Staggered installation of block clusters

During vibratory compaction of the laid blocks, some amount of bedding sand will work its way into the joints between them; the extent of sand getting worked up into the joints will depend on the degree of pre-compaction of sand and the force applied by the block compactor. Standard compactors may have a weight of about 90 kg, plate area of about 0.3 m^2 and apply a centrifugal force of about 15 kN, while heavy duty compactors may weigh 300-600 kg, have a plate area of about $0.5\text{-}0.6 \text{ m}^2$ and apply a centrifugal force of 30-65 kN. Where the bedding sand has been pre-compacted and for heavily

trafficked block pavements, heavy duty compactors should be used. After compaction by vibratory plate compactors, some 2 to 6 passes of a vibratory roller (with rubber coated drums or those of static weight less than 4 tonnes and nominal amplitude of not more than 0.6 mm) will further help in compaction of bedding sand and joint filling.

8.8.4. Joint filling : The importance of complete joint filling cannot be over-emphasised. Unfilled or partially filled joints allow blocks to deflect, leading to loose blocks, possibly spalling the edges and a locally disturbing bedding sand layer, as shown in Fig. 18.

After the compaction of the bedding sand has been completed (and some bedding sand has been forced up in the joints between blocks), the joints should be completely filled with sand meeting the desired specifications, as given in Section 6. The joint filling sand should be stockpiled at suitable locations for convenience. There should be minimum delay in joint filling; the process should in any case, be completed by the end of the day's work.

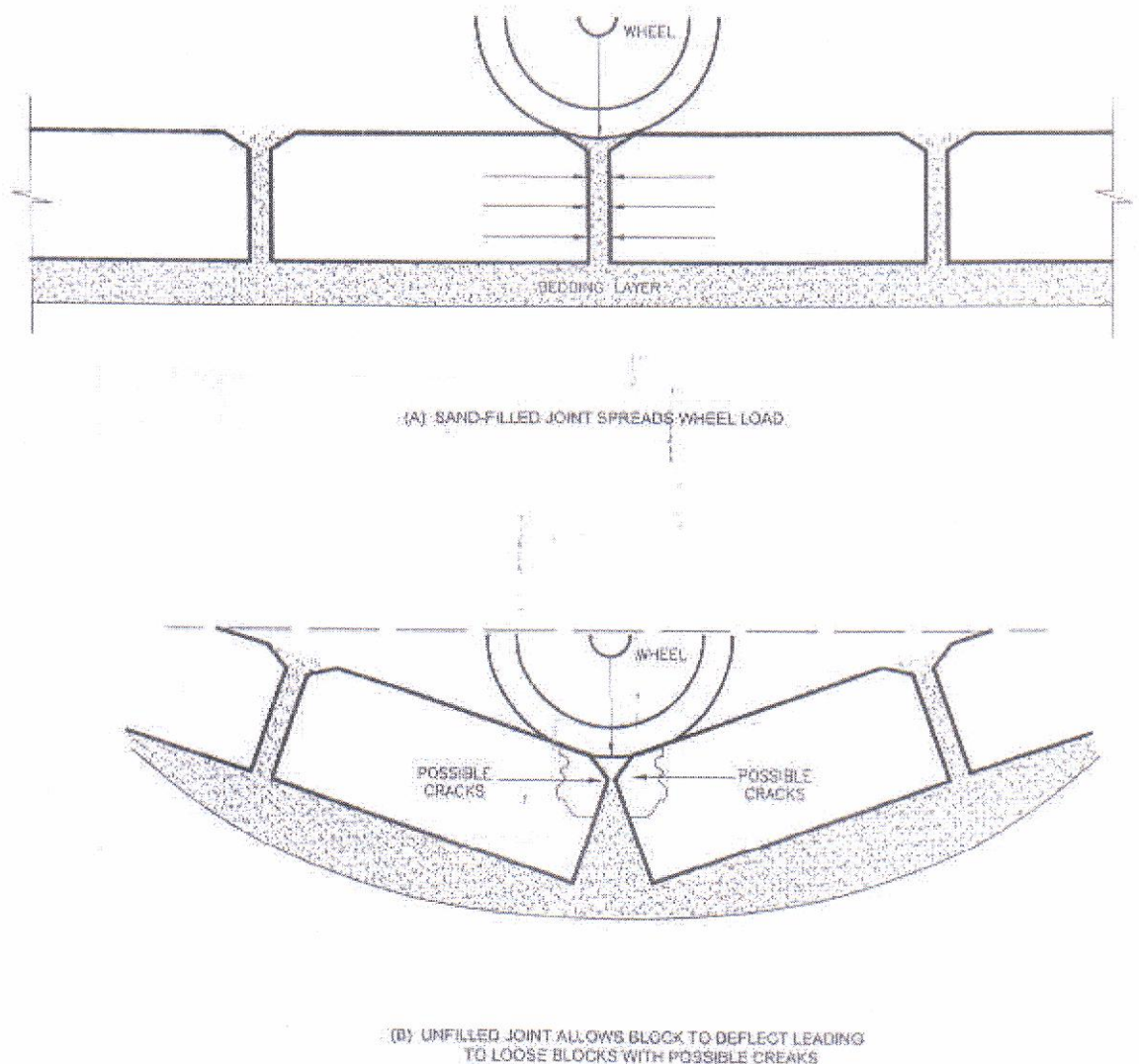


Fig. 18. Need for complete filling of joints

The operation of joint filling comprises of spreading a thin layer of the joint filling sand on the block surface and working the sand into each joint by brooming. Following this, a few passes of heavy plate compactor are applied to facilitate fine sand to fill the joints. The sand should be broomed or spread over the surface with a small surcharge.

Dry sand and dry blocks are best for the filling of joint, as damp sand tends to stick at the very top of the joints; also, if the blocks are wet and the sand dry, the sand will again stick at the joint top. Hence, if either the blocks or sand are wet, one may get a false impression of the joints being full, but the next rain will reveal that they are actually hollow. If the weather does not allow sand and blocks to be dry, the joint filling sand should be washed in by light sprinkling of water. In this case, several cycles of application of sand, water-sprinkling and plate compaction will be necessary to completely fill the joints.

8.8.5. Opening to traffic : Until all the joints are completely filled, no traffic should be permitted over the block pavement. In case of lime or cement treated layers in the pavement, it must be ensured that these are given at least 14 and 7 days respectively to cure, before traffic is permitted. The block pavement should be inspected frequently, to ensure that any incompletely filled joints, exposed by traffic and/or weather are promptly filled. Such frequent inspection should be continued till dust and detritus from the roadway tightens the surface of the joints.

8.8.6. Laying and Surface Tolerances : While the laying, the surface tolerances, given below may be observed:

Layer/Item	Tolerance
Subgrade	+0, -25 mm of nominated level
Select subgrade/Sub-base	+0, -20 mm of nominated level
Base Course	-0, +10 mm of nominated level
Plan deviation	10 mm deviation from a 3 m straight edge
from any 3 m line	10 mm (maximum)
from any 10 m line	20 mm (maximum)
Vertical deviation from 3 m line at kerbs intrusions, channels, edge restraints elsewhere	+3 mm, -0 mm
Maximum difference in surface level between adjacent paving units	+10 mm, -15 mm
Deviation of finished surface level from designated level	+10 mm, -15 mm
Joint width range	2 mm to 4 mm
Percentage of joints outside range	10% max. along 10 m line
Nominal joint width	3 mm

8.8.7. Detailing block pavements : Essentially, there are three important aspects in detailing. These are :

- (i) Curves
- (ii) Treatment of intrusions, and
- (iii) Changes in alignment

8.8.7.1. Curves : It is necessary to cut the paving units to fit the edge restraints. Rectangular blocks of a similar or contrasting colour as an edging have been used to minimise the visual effects of small errors in block cutting. To avoid unsightly and potentially weak construction joints, it is often preferable to change the laying pattern at the curve. For example, as shown in Fig. 19, the curve itself can be installed in herringbone bond and yet the pavement can revert to stretcher bond on the approaches.

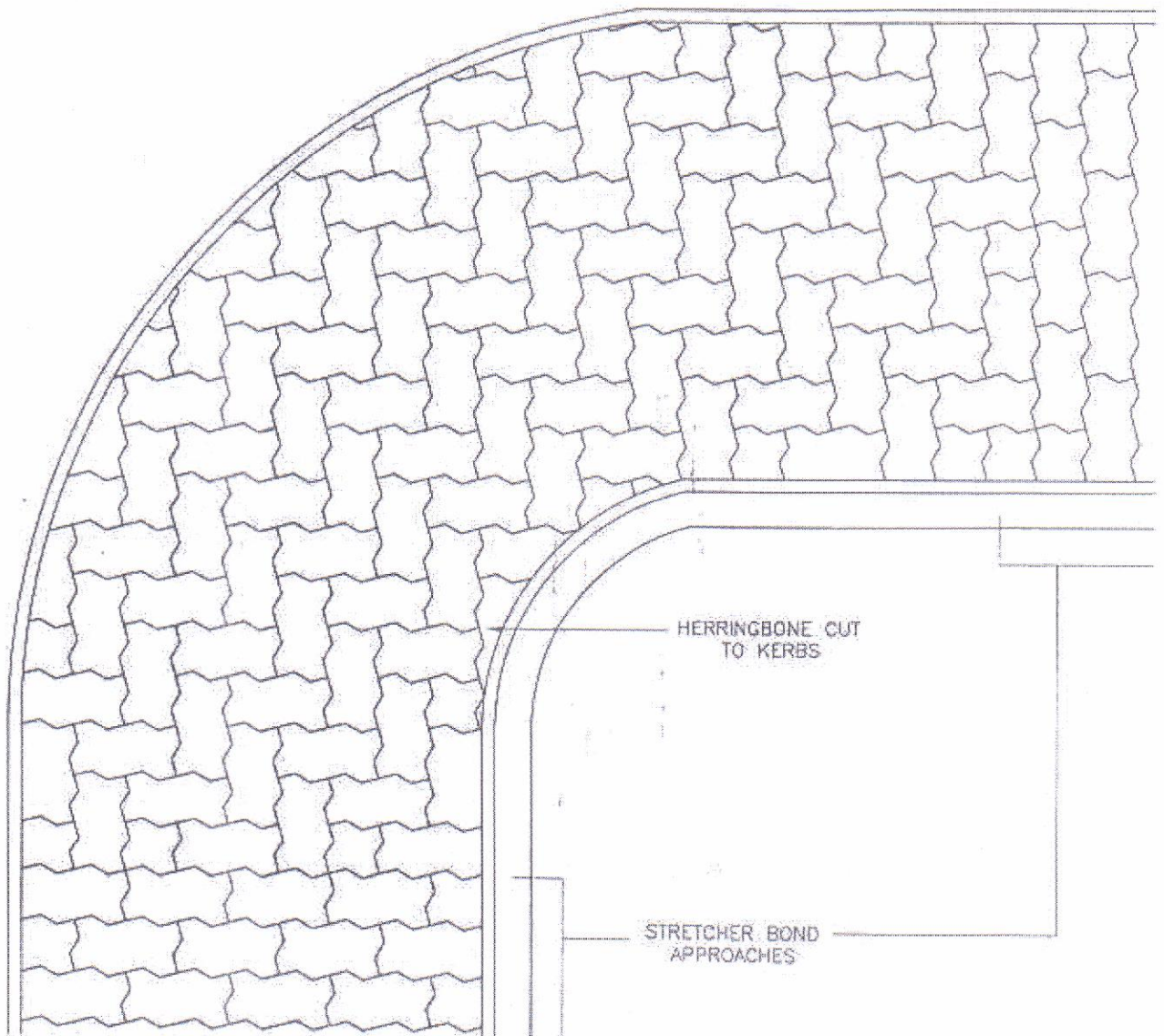


Fig. 19. Curve in herringbone bond and approaches in stretcher bond

8.8.7.2. Pavement intrusions : On some pavements, like in city streets, there could be several intrusions, like, manholes, drainage gulleys, etc. where mating these intrusions with the pavement is desirable. Fig. 20 shows how this should be done around a manhole.

Around intrusions, it is good practice to lay along both sides of the intrusion simultaneously so that closure is made away from the starting workface, rather than carrying the pavement around the intrusion to return to the original laying face (Fig. 20) to avoid accumulation of closing error.

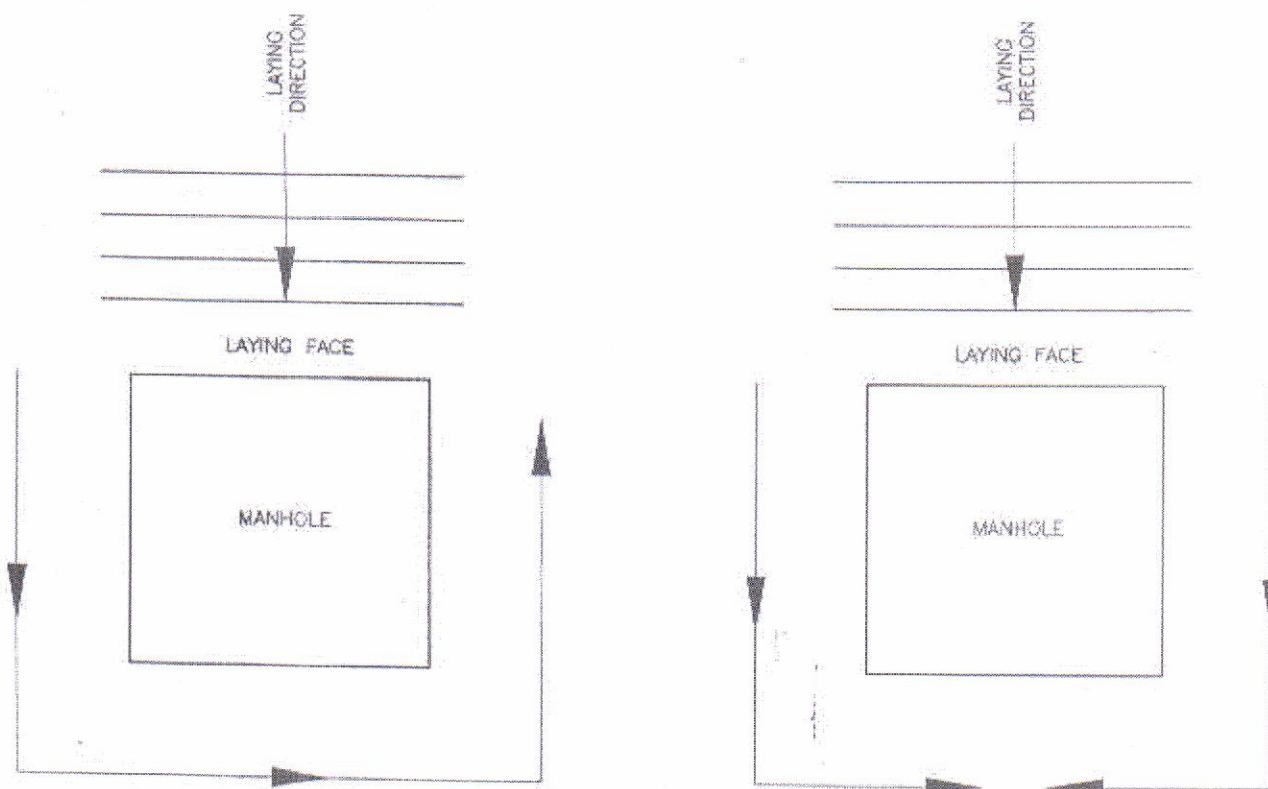


Fig. 20. Laying block paving around a manhole

8.8.7.3. Changes in alignment : Changes in alignment of a road pavement can some times be achieved by the use of special blocks. However, it is generally easier to choose a block that can be installed in herringbone bond and simply cut the blocks to fit the edge restraints. Where aesthetic requirements or shape of the paving unit dictate the use of stretcher bond, then only a 90° shape change in alignment can be achieved without cutting the blocks (Fig. 21). At intersections, if a herringbone bond laying pattern is adopted, the paving can proceed without the need for construction joints (Fig. 22). An alternative to this is to install a shoulder (support) course of rectangular paving units between the main roadway and the side streets; this permits different laying patterns to be used in the two roadways.

8.9. Specifications

Annexure-1 gives the specifications for laying. The BIS Specifications for Precast Concrete Blocks for Paving (under publication) may be followed for the manufacture and testing of blocks.

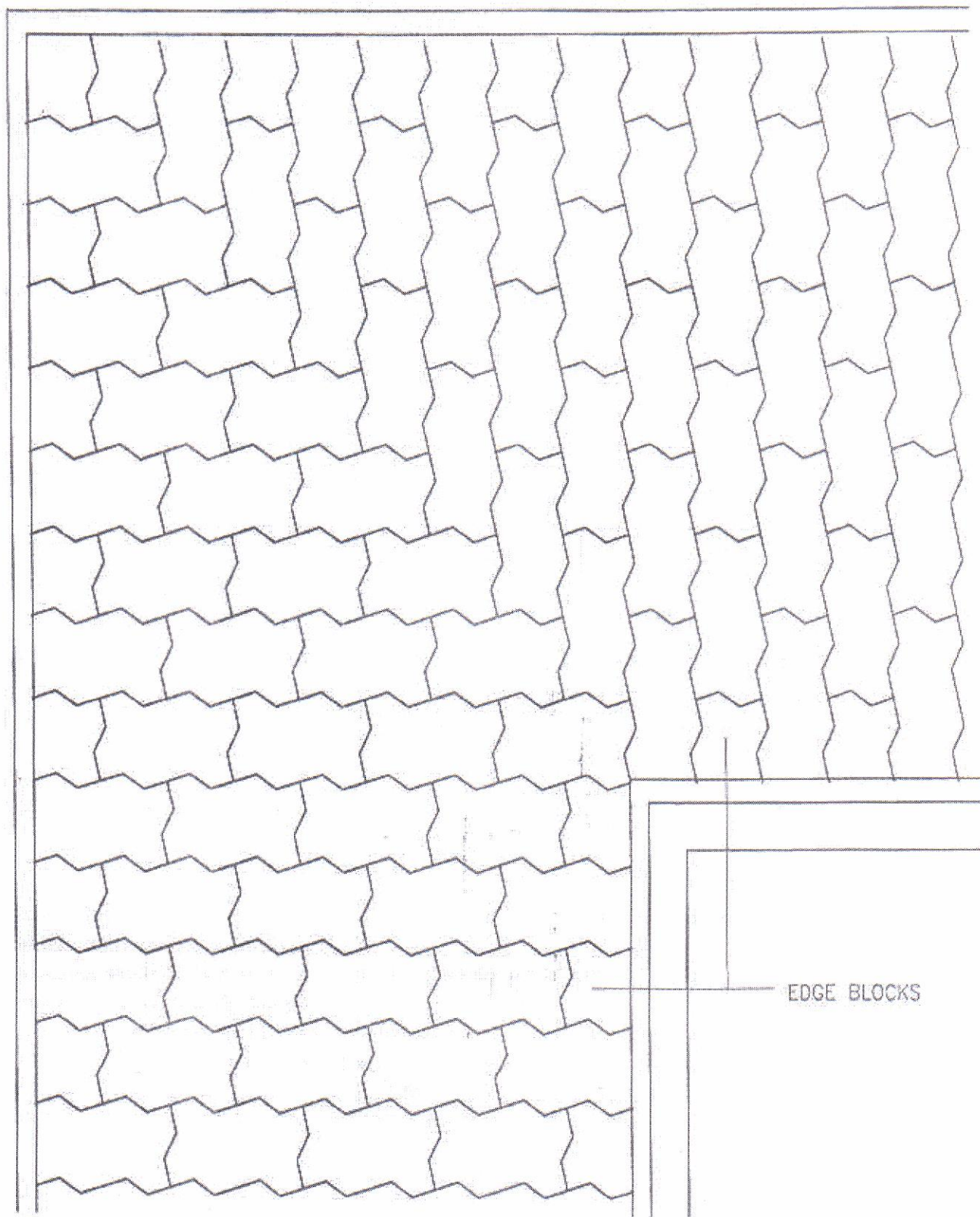


Fig. 21. 90° change in alignment using stretcher bond

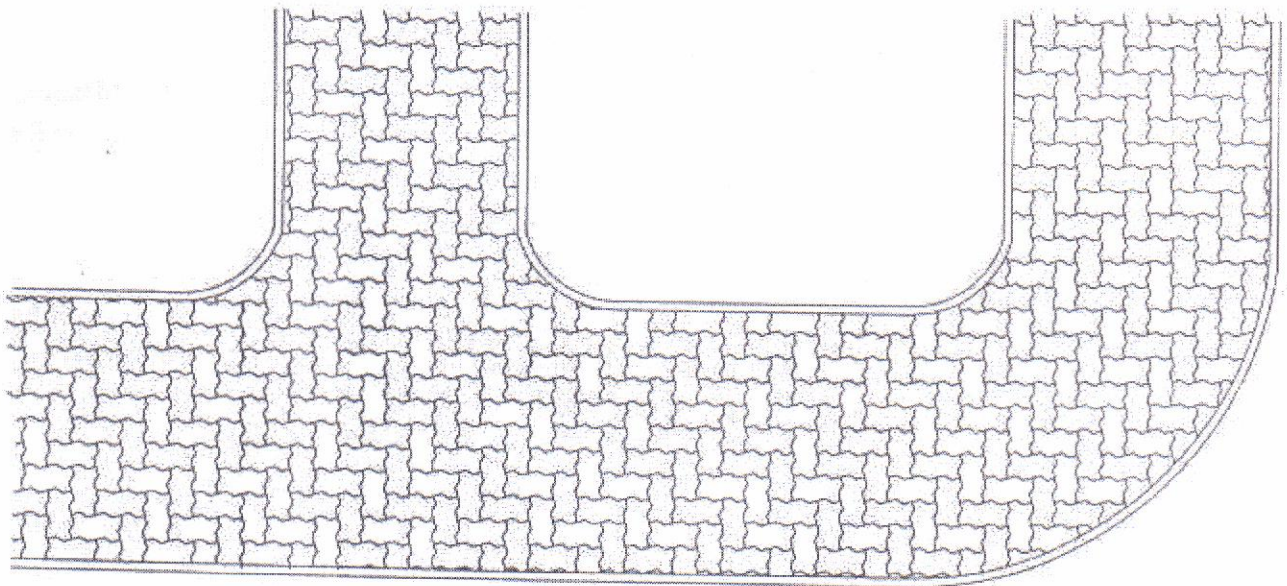


Fig. 22. Adaptation of herringbone bond to changes in alignment

9. MAINTENANCE

9.1. General

Like any other road work, block pavement also should be maintained to give long service. The maintenance requirement of block pavement is minimal. The block pavement requires initial maintenance soon after its laying, say after a week or two for checking sand in the joints. Subsequently, the maintenance is in the form of replacing any damaged block/blocks or raising the settled section, if any. Repair especially after laying a cable duct is much simpler in the case of block pavements. The cut area can be reinstated without any blemish.

9.2. Initial Maintenance

After about a week of laying the blocks there is a need to inspect the surface to check for any loss of sand at joints. Wherever sand level has dropped down it should be reinstated. This type of inspection should continue for two to three months till the sand level is stabilized and topping up is no more required. With time the joints receive fine dust and detritus thus making them waterproof. During rains these joints may allow weeds to grow but these normally should get eliminated with the traffic. In case it does not get eliminated these may have to be controlled by spraying herbicide or by manual removal. Annual inspection, however, will be required.

9.3. Storage of Blocks

For the purpose of reinstating damaged blocks it is necessary to stockpile a small percentage of blocks from the lots used in the construction. The size and colour of the blocks may be difficult to obtain at a later date matching with the original blocks. For important projects, it is normal to stockpile blocks from 1 per cent to 3 per cent initial supply for subsequent use.

9.4. Coating and Cleaning

As part of preventive maintenance, blocks can be sealed using compounds, like, silicone, acrylics and silica flourides for enhancing the colour, reducing absorptive nature of the blocks and for improving surface toughness. These coating have life of 1 to 3 years and hence they have to be repeated as per the requirement. The most durable of these chemicals is solvent-borne acrylics which are abrasion resistant and also minimize chemical effects of spillage even at 60°C.

Cleaning of block pavement can be done by mechanical brooms, compressors or even by manual means. For removing certain stains, chemicals, like, oxalic, acetic and phosphoric acids etc. are used. Sometimes it may be expedient to replace the blocks where stains have penetrated to a greater depth.

1. TECHNICAL SPECIFICATIONS FOR LAYING CONCRETE PAVING BLOCKS

1.1. Base

1.1.1. The Finished surface of the concrete base shall match the design profile of the concrete blocks within ± 10 mm.

1.1.2. Compaction shall be done with vibratory roller. In restricted areas where normal rollers cannot operate, hand-held or plate vibrators should be employed.

1.2. Bedding Sand Layer

1.2.1. The bedding sand layer shall be from either a single source or blended to achieve the following grading.

<i>IS Sieve Size</i>	<i>Per cent Passing</i>
9.52 mm	100
4.75 mm	95-100
2.36 mm	80-100
1.18 mm	50-95
600 micron	25-60
300 micron	10-30
150 micron	0-15
75 micron	0-10

Single sized, gap-graded sands or those containing an excessive amount of fines will not be used. The sand particles should preferably be angular type.

The joint-filling sand should pass a 2.35 mm sieve and be well graded. The following grading is recommended:

<i>Sieve Size</i>	<i>Per cent Passing</i>
2.36 mm	100
1.18 mm	90-100
600 micron	60-90
300 micron	30-60
150 micron	15-30
75 micron	0-10

The use of cement in the joint-filling sand is not recommended as a general practice as the cemented sand is likely to crack into segments which are easily dislodged.

1.2.2. Average thickness of this laying course shall be 20 to 40 mm.

1.2.3. The sand should be slightly moist, and the moisture content shall be about 4 per cent by weight.

1.2.4. It should contain not more than 3 per cent by weight of clay and silt and the materials shall be free from deleterious salts or contaminates.

1.2.5. The finished surface of the bedding layer shall match exactly the design profile as indicated on the drawings.

1.2.6. Before placing the bedding layers, the surface of concrete should be cleared by sweeping.

1.2.7. Walking or driving on the finished surface of the bedding layer shall not be permitted.

1.3. Concrete Paving Blocks

1.3.1. Laying of the blocks shall be done, precisely at the indicated level and profile and in a way that a good surface draining to the gulley chambers is assured.

1.3.2. Around gulley chambers and inspection pits the pavement shall have a level of 5 mm higher than the above mentioned elements.

1.3.3. The blocks shall be laid to the pattern directed by the Engineer or the pattern recommended by the designer. The blocks shall be laid as tight as possible to each other. The maximum joint width shall be limited to 4 mm.

1.3.4. Laying of broken blocks is not allowed except along connections or edges. The maximum length of a purpose broken block is 100 mm. Breaking of the blocks shall be done with a "block splitter" or a mechanical saw.

1.3.5. Fine angular sand as per specification shall be brushed into the joints, and thereafter compaction shall be done with a vibrating plate compactor on a clean surface. After compaction, again fine angular sand shall be brushed into the joints.

1.4. Surface Tolerances

1.4.1. Surface tolerance for finished surface shall be ± 10 mm from the design level.

1.4.2. The surface tolerance for base course shall be in the range of 0 to +10 mm from nominated level and 10 mm deviation from a 3 m straight edge.

1.4.3. The surface tolerance for sub-base shall be within 0 to -20 mm of nominated level.

2. FIELD/LABORATORY TESTS

(a) Necessary field/laboratory tests shall be carried out by the contractor while executing the work at his own cost.

(b) The field/laboratory tests may be conducted in an Engineering College/ approved technical institution as directed by the Engineer.